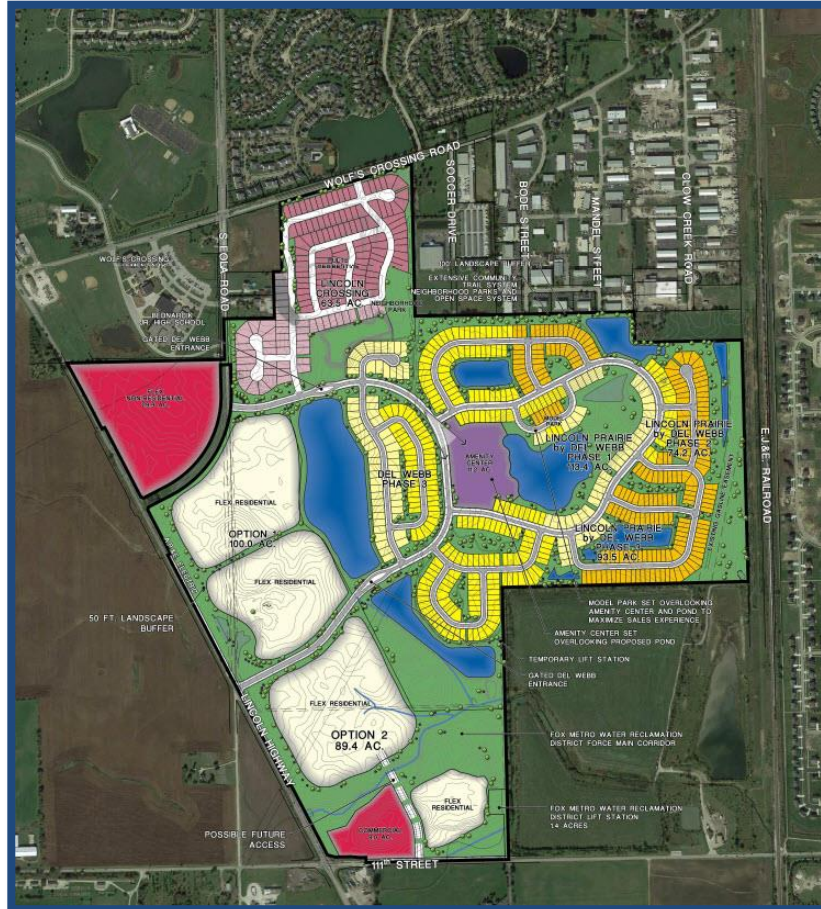


Traffic Impact Study Proposed Lincoln Prairie Mixed-Use Development

Aurora, Illinois



Prepared For:

Pulte Homes



March 19, 2021

1. Introduction

A traffic impact study was conducted for the proposed Lincoln Prairie, a mixed-use development to be located on the eastside of Lincoln Highway (US Route 30) between Wolf's Crossing and 111th Street in Aurora, Illinois. The site, which is currently vacant, is generally bounded by Wolf's Crossing Road to the north, Lincoln Highway to the west, 111th Street to the south and an E.J.&E Railroad to the east. The plans call for single-family homes, the Del Webb Active Adult Community, and approximately 39 acres of commercial development.

The overall plans of the proposed Lincoln Prairie development call for the active adult homes to be located on the eastern side of the site, single-family homes on the north side of the site, two commercial lots on the north and south side of the site fronting Lincoln Highway and approximately 95.3 acres of flex residential land to be developed with either active adult homes or single-family homes (assumed to be developed with 238 single-family homes for analysis purposes). Overall, the entire development proposes the following land uses and densities:

- Active Adult – 548 homes
- Traditional Residential – 162 single-family homes
- Flex Residential – 369 single-family homes
- Commercial – 247,000 square feet

Access to the development will be provided off Lincoln Highway, Eola Road, Wolf's Crossing Road, and 111th Street.

In 2019, the Illinois Department of Transportation (IDOT) improved the intersection of Lincoln Highway at Eola Road. Which included the realignment of Eola Road at Lincoln Highway, signalizing the intersection and providing a northbound right turn lane and a southbound left-turn lane.

The purpose of this study was to examine background traffic conditions, assess the impact that the proposed development will have on traffic conditions in the area, and determine if any roadway or access improvements are necessary to accommodate traffic generated by the proposed development. The sections of this report present the following:

- Existing roadway conditions including vehicle, pedestrian, and bicycle traffic volumes for the weekday morning and weekday evening peak hours
- A description of the proposed development
- Vehicle trip generation for the proposed development
- Directional distribution of development-generated traffic
- Regional growth in traffic for Year 2036 no-build conditions
- Future transportation conditions including access to and from the development

Traffic capacity analyses were conducted for the weekday morning and evening peak hours for the following conditions:

1. Year 2021 Base Conditions – Analyzes the capacity of the existing roadway system using peak hour traffic volumes conducted in 2021 adjusted to represent pre-pandemic conditions.
2. Year 2036 No-Build Conditions – Analyzes the capacity of the existing roadway system using Year 2021 base traffic volumes increased by an ambient area growth factor not attributable to any particular development.
3. Year 2036 Total Projected Conditions – Analyzes the capacity of the future roadway system using the projected traffic volumes that include the Year 2020 base traffic volumes, ambient area growth not attributable to any particular development, and the traffic estimated to be generated by the proposed development.

2. Existing Conditions

Existing traffic and roadway conditions were documented based on field visits and traffic counts conducted by KLOA, Inc. The following provides a detailed description of the physical characteristics of the roadways including geometry and traffic control, adjacent land uses, and peak hour traffic flows along area roadways.

Site Location

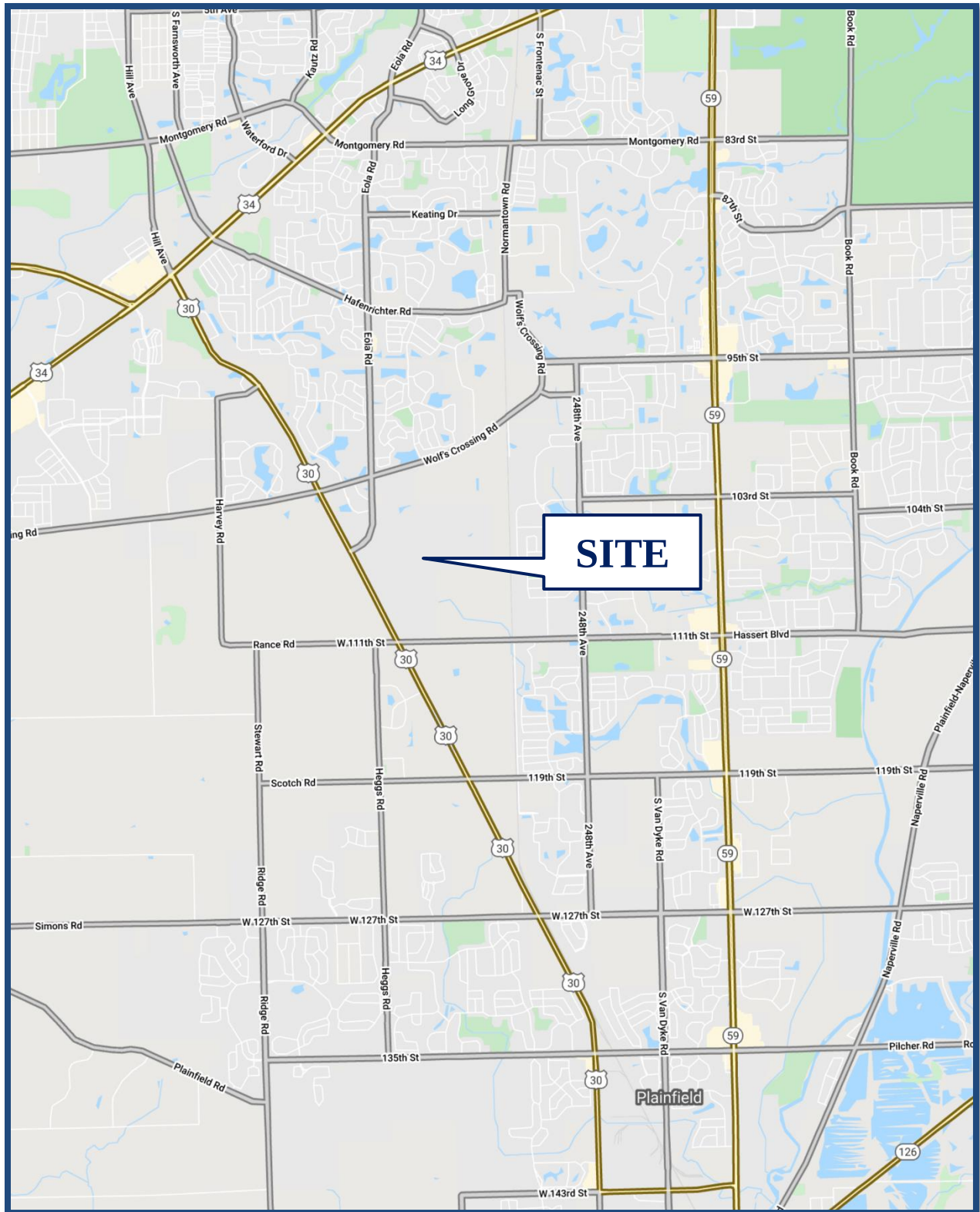
The site, which is currently vacant, is generally bounded by Wolf's Crossing Road to the north, Lincoln Highway to the west, 111th Street to the south and an E.J.&E Railroad to the east. Land uses in the vicinity of the site include the Amber Fields and Lakewood Valley Residential Subdivisions and the Clow Creek Industrial Park and other industrial uses north of the site, the Ashwood Park residential subdivision to the east, and the Sunny Farm Acres and Auburn Lakes residential subdivisions to the south. The Wolf's Crossing Elementary School and Bednarcik Jr. High School are located in the southwest quadrant of the intersection of Wolf's Crossing Road and Eola Road with access on Lundquist Drive which extends between the two roadways.

Figure 1 shows the location of the site in relation to the area roadway system. **Figure 2** shows the conceptual plan on an aerial.

Existing Roadway System Characteristics

The characteristics of the existing roadways that surround the proposed development are illustrated in **Figure 3** and described below.

Lincoln Highway (U.S. Route 30) is a northwest-southeast, other principal arterial roadway that generally provides one lane in each direction. At its signalized intersection with Wolf's Crossing Road, Lincoln Highway provides an exclusive left-turn lane and a shared right-turn/through lane on both approaches. At its signalized intersection with Eola Road, Lincoln Highway provides a through lane and an exclusive right-turn lane on the northbound approach and an exclusive left-turn lane and a through lane on the southbound approach. At its signalized intersection with 111th Street, Lincoln Highway provides an exclusive left-turn lane and a shared through/right-turn lane on both approaches. Lincoln Highway is under the jurisdiction of IDOT, is not designated as a Strategic Regional Arterial (SRA) route, is designated a class II truck route, and is part of the National Highway System (NHS). Lincoln Highway carries an annual average daily traffic (AADT) of 16,800 vehicles north of Wolf's Crossing Road and 20,000 vehicles south of Wolf's Crossing Road (IDOT 2019) and has posted speed limit of 50 mph north of Eola Road and 55 mph south of Eola Road.



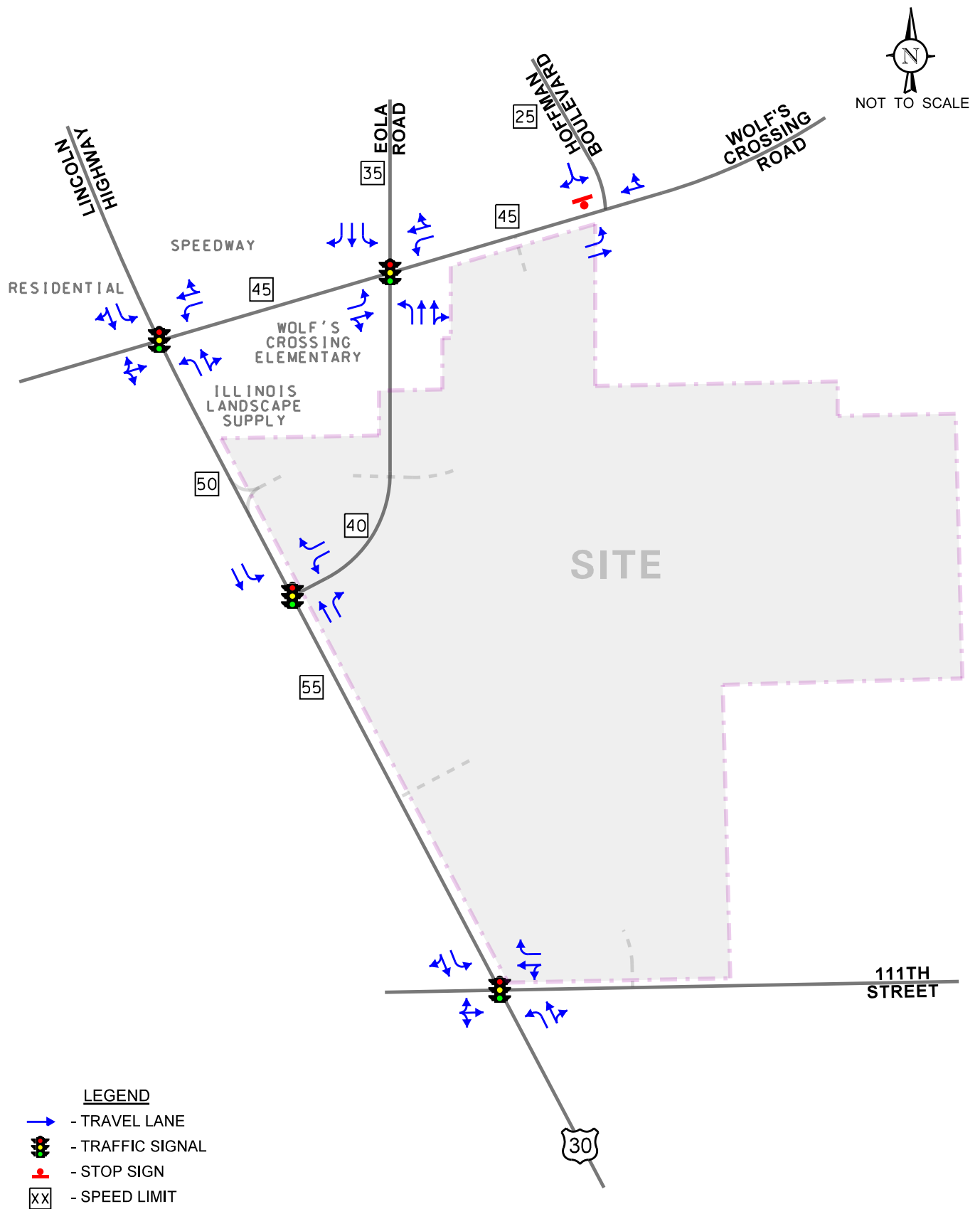
Site Location

Figure 1



Aerial View of Conceptual Plan

Figure 2



Wolf's Crossing Road is an east-west, minor arterial roadway that generally provides one lane in each direction divided by a striped median. At its signalized intersection with Lincoln Highway, Wolf's Crossing Road provides a shared left-turn/through/right-turn lane on the eastbound approach and an exclusive left-turn lane and a shared through/right-turn lane on the westbound approach. At its signalized intersection with Eola Road, Wolf's Crossing Road provides an exclusive left-turn lane and a shared through/right-turn lane on both approaches. At its unsignalized intersection with Hoffman Boulevard, Wolf's Crossing Road provides one through lane in each direction and an exclusive eastbound left-turn lane. Wolf's Crossing Road is under the jurisdiction of the City of Aurora, is not designated as an SRA route, carries an AADT of 8,650 vehicles (IDOT 2019), and has a posted speed limit of 45 mph. Wolf's Crossing Road has a posted weight limit of 12 tons east of Lincoln Highway and a six-ton weight limit per axle west of Lincoln Highway.

Eola Road is a north-south collector roadway that generally provides two lanes in each direction north of Wolf's Crossing Road and one lane in each direction south of Wolf's Crossing Road. In 2019, the Illinois Department of Transportation (IDOT) improved the intersection of Lincoln Highway at Eola Road. The improvement involved realigning Eola Road to intersect Lincoln Highway at a perpendicular angle and signalizing the intersection. At this intersection, Eola Road terminates and provides an exclusive left-turn lane and an exclusive right-turn lane. At its signalized intersection with Wolf's Crossing Road, Eola Road provides an exclusive left-turn lane, a through lane, and a shared through/right-turn lane on the northbound approach and an exclusive left-turn lane, a through lane, and an exclusive right-turn lane on the southbound approach. Eola Road is under the jurisdiction of the City of Aurora and is designated as an SRA (WIKADUKE) route. Eola Road carries an AADT of 9,700 vehicles north of Wolf's Crossing Road and 3,900 vehicles south of Wolf's Crossing Road (IDOT 2019) and has posted speed limit of 35 mph north of Wolf's Crossing Road and 40 mph south of Wolf's Crossing Road.

111th Street is an east-west roadway that generally provides one lane in each direction and is designated as a minor arterial roadway east of Lincoln Highway and as a major collector roadway west of Lincoln Highway. At its signalized intersection with Lincoln Highway, 111th Street provides a shared left-turn/through/right-turn lane on eastbound approach and a shared through/left-turn lane and an exclusive right-turn lane on the westbound approach. 111th Street is under the jurisdiction of Wheatland Township, is not designated as an SRA route, carries an AADT of 15,100 vehicles (IDOT 2019) and has posted speed limit of 45 mph.

Hoffman Boulevard is a north-south local roadway that extends north from Wolf's Crossing Road and provides one lane in each direction. At its unsignalized intersection with Wolf's Crossing Road, Hoffman Boulevard provides a shared left-turn/right-turn lane on the southbound approach. Hoffman Boulevard is under the jurisdiction of the City of Aurora and has posted speed limit of 25 mph.

Year 2021 Base Traffic Volumes

In order to determine current traffic conditions within the study area, KLOA, Inc. conducted peak period traffic vehicle, pedestrian, and bicycle movement traffic counts utilizing Miovision Scout Collection Units at the following intersections:

- Lincoln Highway with Wolf's Crossing Road
- Lincoln Highway with Eola Road
- Lincoln Highway with 111th Street
- Wolf's Crossing Road with Eola Road
- Wolf's Crossing Road with Hoffman Boulevard

The traffic counts were conducted on Thursday, January 7, 2021 during the weekday morning (6:00 A.M. to 9:00 A.M.) and weekday evening (4:00 P.M. to 6:00 P.M.) peak periods. The results of the traffic counts show that the peak hours of traffic generally occur between 7:30 A.M. and 8:30 A.M. during the weekday morning peak period and between 4:30 P.M. and 5:30 P.M. during the weekday evening peak period. Copies of the traffic count summary sheets are included in the Appendix. In order to present pre-pandemic Year 2021 conditions, the traffic volumes were adjusted (increased by approximately 35 percent) based on the Intersection Design Study (IDS) prepared by IDOT for the intersection of Lincoln Highway with Eola Road and on past traffic projections at the intersection of Lincoln Highway with Wolf's Crossing Road.

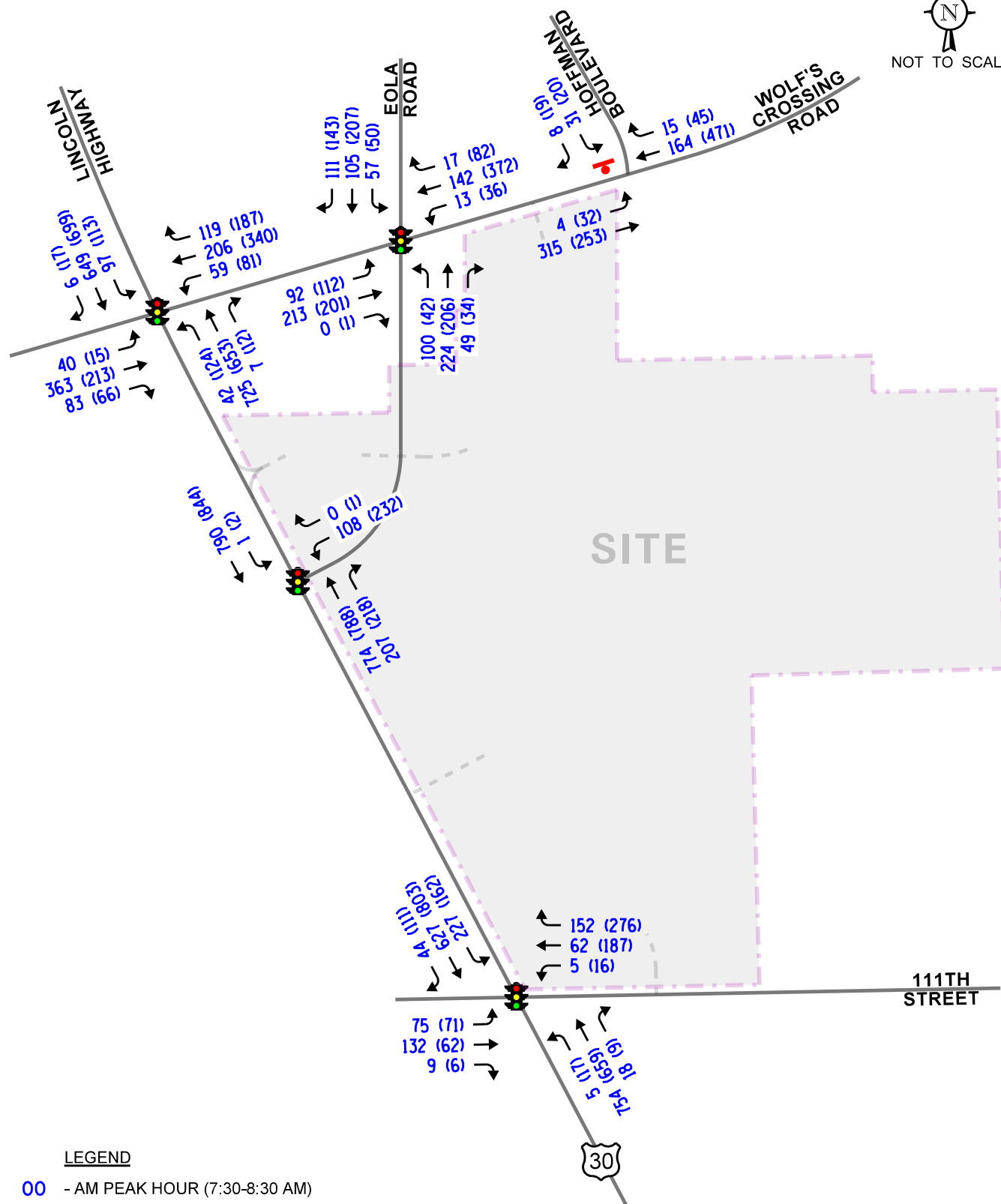
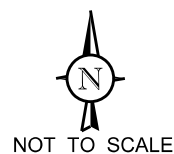
The Year 2021 adjusted traffic volumes are illustrated in **Figure 4**.

Proposed Regional Improvements

The Village of Oswego has conducted a Pre-Phase I study for the Wolf's Crossing Road corridor from U.S. Route 34 east to Eola Road. In addition to widening Wolf's Crossing Road to provide four through lanes, the following improvements were also identified for the intersection of U.S. Route 30 with Wolf's Crossing Road:

- Widen Wolf's Crossing Road to provide an exclusive left-turn lane, two through lanes and an exclusive right-turn lane on both approaches.
- Widen U.S. Route 30 to provide two through lanes in each direction. The existing exclusive left-turn lanes on both approaches will be maintained.
- Modify the existing traffic signal to provide protected-permissive left-turn phases to the east-west approaches as well as right-turn overlaps.

Based on discussions with the Village of Oswego, all of these improvements will require substantial right-of-way acquisition and no timeframe or funding has been allocated to these improvements.



3. Traffic Characteristics of the Proposed Development

To evaluate the impact of the subject development on the area roadway system, it was necessary to quantify the number of vehicle trips the site will generate during the respective two peak hours and then determine the directions from which the proposed traffic will approach and depart the site.

Proposed Site and Development Plan

Lincoln Prairie is a proposed mixed-use development of residential and commercial land uses to be located in the northeast quadrant of the intersection of 111th Street with Lincoln Highway in Aurora, Illinois. The overall plan calls for the site to be developed as follows:

- The Del Webb active adult community on the eastern side of the site. As proposed, the community will consist of 548 homes and a central amenity center with access provided via a proposed access road on Eola Road and a full access on Lincoln Highway.
- A residential subdivision on the northern side of the site. As proposed the subdivision will consist of 162 family home with access provided via a connection to the proposed full access road off Eola Road and via a proposed full movement access drive on Wolf's Crossing Road.
- A 29.9-acre commercial outlot parcel in the northeast corner of Lincoln Highway and Eola Road. Based on preliminary plans, the parcel was assumed to be developed with 157,000 square feet of commercial/retail space with access provided via a restricted access drive on Lincoln Highway and a full movement access drive on Eola Road.
- A 9.0-acre commercial outlot parcel in the northeast corner of the intersection of Lincoln Highway and 111th Street. The parcel was assumed to be developed with 90,000 square feet of commercial/retail space with access provided via a connection to a proposed full movement access road on 111th Street.
- 147.2 acres of flex residential parcels to be developed with either active adult homes or single-family homes. In order to provide a conservative analysis, the parcels were assumed to be developed with single family homes at a density of 2.5 dwelling units per acre (369 total units). Access to Flex parcels were assumed via full access movements to Eola Road, Lincoln Highway and 111th Street.

A copy of the proposed site plan is included in the Appendix.

Development Access

Access to the development will be provided via the following access roads:

Wolf's Road Crossing Full Movement Access Road. To the north, Lincoln Crossing will be access via a full movement access road will extend south from Wolf's Crossing Road approximately 1,300 feet east of Eola Road ("Wolf Access Road"). Wolf Access Road will provide one inbound lane and one outbound lane with outbound movements under stop sign control. Wolf Access Road will extend south through Lincoln Crossing, connect to Eola Access Road (hereinafter defined) and thereby provide a southern point of access for Lincoln Crossing. Wolf's Crossing Road should be restriped to provide a westbound exclusive left-turn lane.

Lincoln Prairie by Del Webb Proposed Access Road. The heart of the Lincoln Prairie will be served by a two-lane access road that connects to Eola Road to the north and Lincoln Highway to the south. The central section of this access road, located between guard houses for the Lincoln Prairie by Del Webb subdivision, will be owned and maintained as private road. The portions of the access road located outside of the guard houses (respectively the "Eola Access Road" and "Lincoln Access Road), will be dedicated and maintained as public roads.

Eola Access Road. The Eola Access Road is a full movement access located approximately 1,875 feet south of Wolf's Crossing Road which extends east to the guard house for Lincoln Prairie by Del Webb. Eola Access Road will provide one inbound lane and two outbound lanes striped to provide an exclusive left-turn lane and an exclusive right-turn lane. Eola Access Road will serve Lincoln Crossing to the north and Lincoln Prairie by Del Webb to the east. It is anticipated that future access to Commercial Parcel 1 will align with Eola Access Road. At the Eola Access Road, Eola Road will be widened to provide an exclusive northbound right-turn lane and an exclusive southbound left-turn lane. The right-turn lane will provide 115 feet of storage and a 150-foot taper. The left turn lane will provide 115 feet of storage and a 156-foot taper. The Eola Access Road and corresponding improvements to Eola Road should be constructed as part of the first phase of Lincoln Prairie by Del Webb.

Lincoln Access Road. The Lincoln Access Road is a full movement access located approximately 2,110 feet south of Eola Road which extends east to the gaurad house for Lincoln Prairie by Del Webb. Lincoln Access Road will serve the Flex Parcels to the north and south and Lincoln Prairie by Del Webb to the east. Lincoln Access Road will provide one inbound lane and two outbound lanes striped to provide a left-turn and a right-turn lane. At Lincoln Access Road, Lincoln Highway will be widened to provide an exclusive northbound right-turn lane and an exclusive southbound left-turn lane. These turn lanes, based on IDOT's Bureau of Design and Environment (BDE) Manual, should provide 240 feet of storage and 240-foot taper. Outbound movements from the access road will be under stop sign control. Lincoln Access Road and corresponding improvements to Lincoln Highway should be constructed as part of the second phase of Lincoln Prairie by Del Webb.

Lincoln Highway Restricted Access Drive. A restricted right-in/right-out access drive to Lincoln Highway is anticipated approximately 820 feet north of Eola Road. This restricted access will exclusively serve Commercial Parcel 1. This access drive will provide one inbound lane and one outbound lane channelized and signed to prevent left-turn movements. Lincoln Highway should be widened to provide an exclusive northbound right-turn lane. This turn lane should provide 240 feet of storage a 240-foot taper. Outbound movements from the access drive will be under stop sign control. The restricted access improvements should be constructed as part of the development of Commercial Parcel 1.

111th Street Full Movement Access Road. A full movement access road will be provided on 111th Street approximately 900 feet east of Lincoln Highway (“111th Access Road”). 111th Access Road will provide one inbound lane and two outbound lanes. 111th Access Road will serve both Commercial Parcel 2 and Flex Parcel 2. At 111th Access Road, 111th Street should be widened to provide an eastbound exclusive left-turn lane. Outbound movements from the access drive will be under stop sign control. The 111th Access Road should be constructed with the development of Flex Parcel 2 or Commercial Parcel 2, whichever occurs first.

Eola Road Improvements

It is important to note that Phase I of the development involves the construction of Lincoln Prairie by Del Webb and the Lincoln Crossing single-family homes fronting Wolf’s Crossing. In Phase I and based on the projected traffic volumes, a three -lane cross-section on Eola Road will be sufficient to accommodate the projected traffic volumes. However, once the commercial and the flex residential parcels are developed, the road should be widened to provide two lanes in each direction and a traffic signal should be provided (to be discussed later on) at the intersection of Eola Road and the access road to the Active Adult community and the commercial parcel.

Directional Distribution of Development Traffic

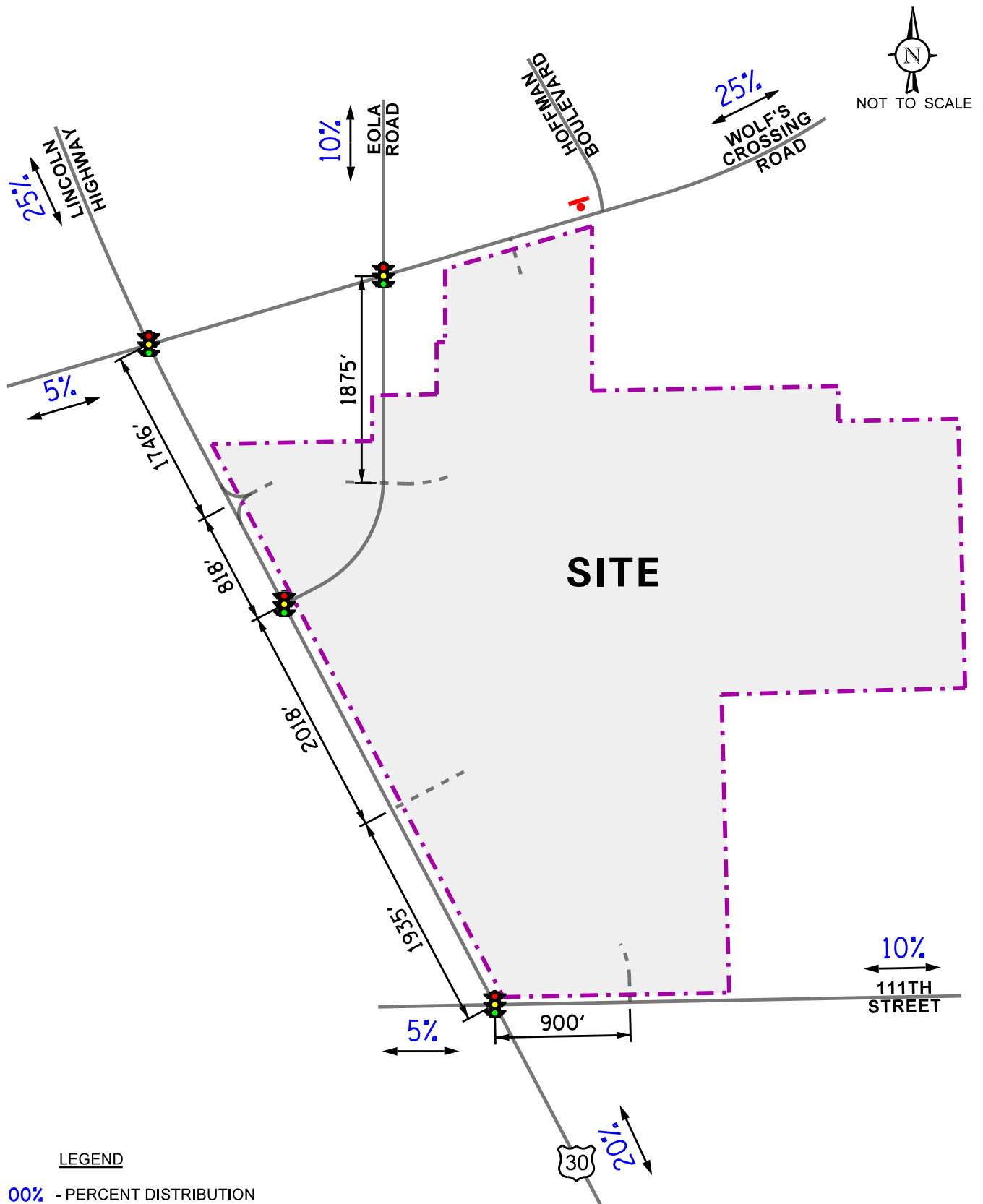
The directions from which traffic will approach and depart the development were estimated based on existing travel patterns, as determined from the existing traffic counts. **Figure 5** illustrates the directional distribution of the traffic projected to be generated by the proposed development. Figure 5 also shows the distance, in feet, between the existing and proposed access intersections.

Development Traffic Generation

The number of peak hour vehicle trips estimated to be generated by the proposed development was based on vehicle trip generation rates contained in *Trip Generation Manual*, 10th Edition, published by the Institute of Transportation Engineers (ITE). Land-use code 251 (Senior Adult Housing – Detached) was used for the Del Webb active adult community, land-use code 210 (Single-Family Detached Housing) was used for the Pulte Homes residential subdivision and flex residential land, and Land-use code 820 (Shopping Center) was used for the commercial developments. Copies of the ITE trip generation sheets are included in the Appendix. It should be noted that it is expected that some of the patrons of the commercial developments will be residents of the active adult community, residential subdivision and the flex residential land. Further, it is expected that number of trips made to the commercial developments will be diverted from the existing traffic on the area roadways and will not be new to area. However, to provide a conservative analysis, no reduction was taken to account for pass-by traffic or interaction between developments. **Table 1** shows the estimated vehicle trip generation for the weekday morning and weekday evening peak hours as well as the weekday daily two-way traffic volumes for the overall development. Inspection of Table 1 indicates that approximately 60 percent of the total site traffic will be generated by the commercial parcels.

Table 1
ESTIMATED VEHICLE TRIP GENERATION FOR PROPOSED DEVELOPMENT

ITE Land-Use Code	Type/Size	Weekday Morning Peak Hour			Weekday Evening Peak Hour			Daily Two-Way Trips
		In	Out	Total	In	Out	Total	
251	Senior Adult Housing - Detached (548 Homes) ¹	49	100	149	110	71	181	2,514
210	Single Family Detached Housing (162 Homes) ²	30	90	120	101	60	171	1,620
820	Shopping Center (247,000 s.f)	170	105	275	509	552	1,061	11,120
210	Single Family Detached Housing (369 Homes) ³	67	200	267	224	132	356	3,456
Total		316	495	811	944	815	1,759	18,710
1 – Del Webb Community								
2 – Pulte Homes subdivision (north side of the site)								
3 – Flex Residential Parcels								



4. Projected Traffic Conditions

The total projected traffic volumes include the existing traffic volumes increased by a regional growth rate and the traffic estimated to be generated by the proposed subject development.

Development Traffic Assignment

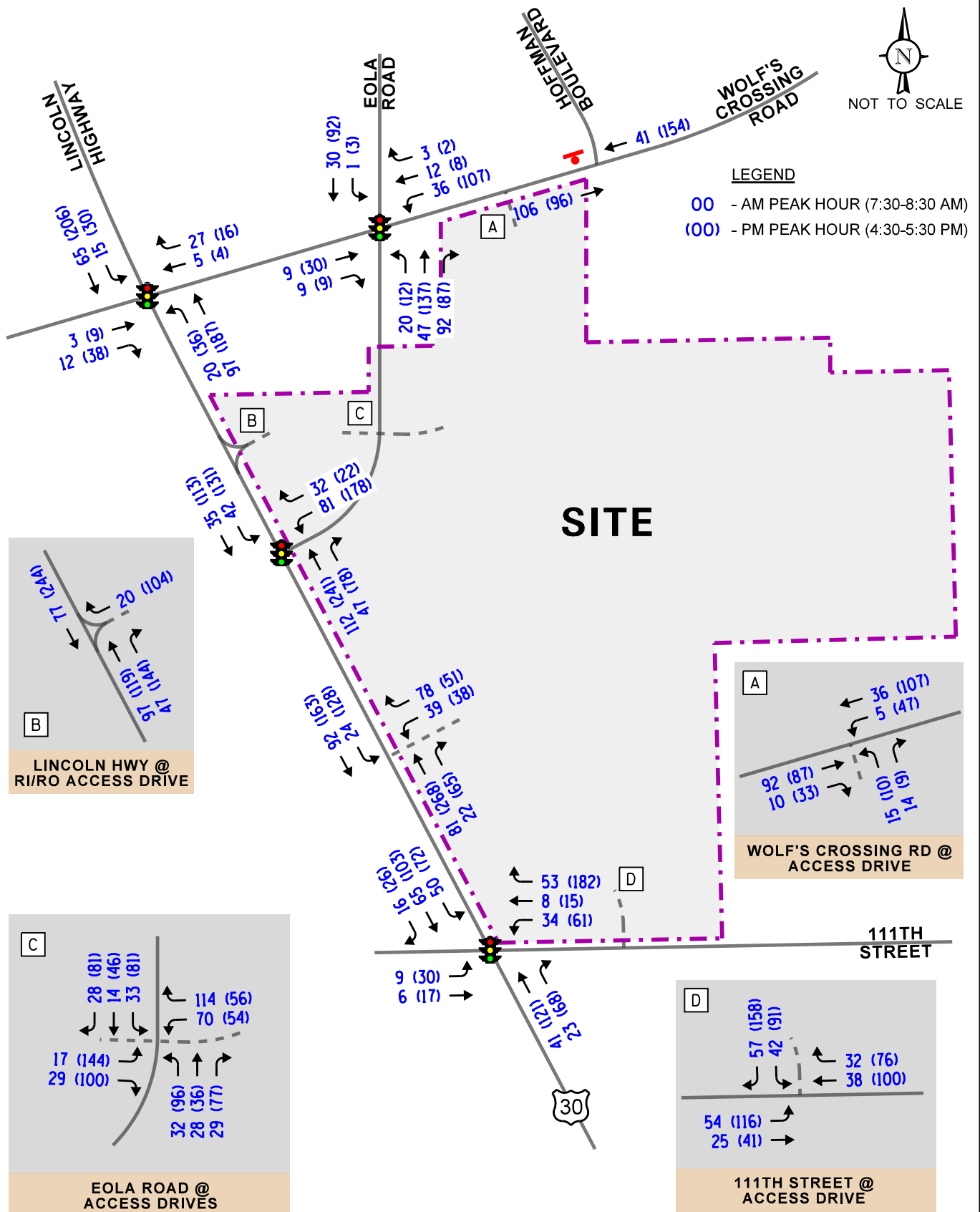
The estimated weekday morning and evening traffic volumes that will be generated by the proposed development were assigned to the roadway system in accordance with the previously described directional distribution (Figure 5). The site traffic assignment for the proposed development is illustrated in **Figure 6**.

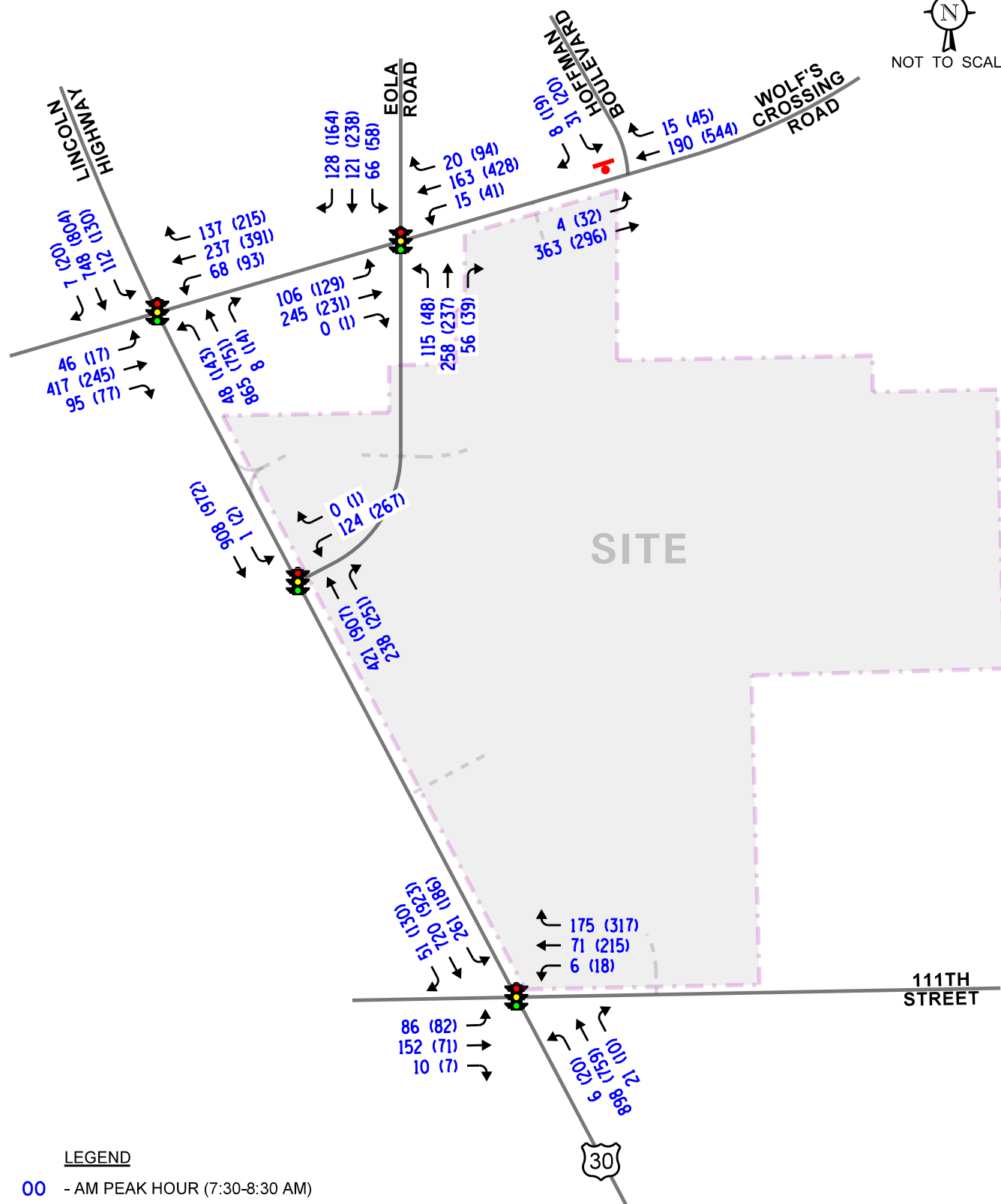
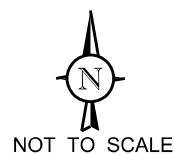
Background (No-Build) Traffic Conditions

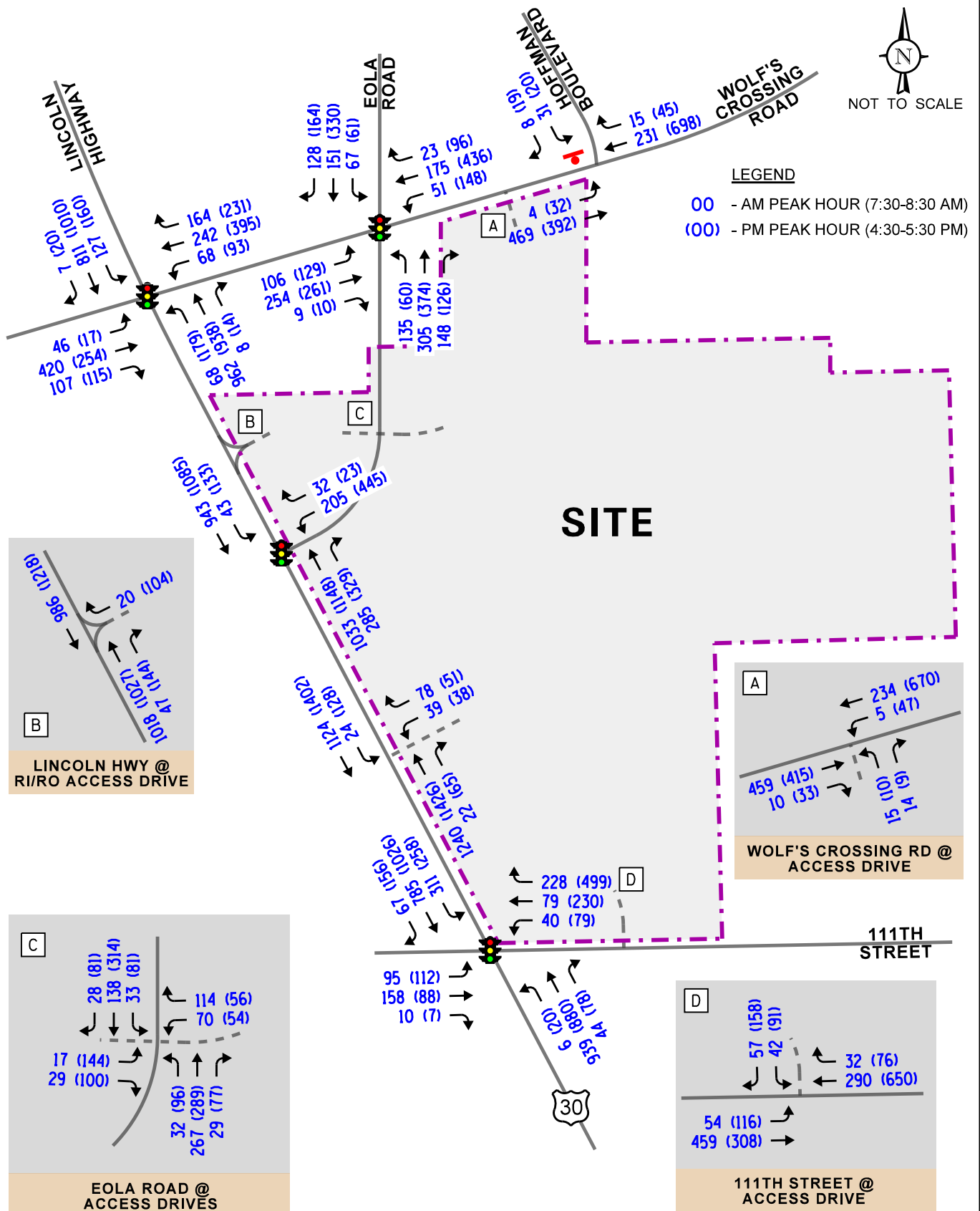
The Year 2021 base traffic volumes (Figure 4) were increased by a regional growth factor to account for the increase in existing traffic related to regional growth in the area (i.e., not attributable to any particular planned development). Based on AADT projections provided by the Chicago Metropolitan Agency for Planning (CMAP), the existing traffic volumes were increased by an annually compounded growth rate of 0.9 percent per year for 15 years (buildout year plus five years) for a total of 15 percent. The projected Year 2036 no-build traffic volumes, which include the adjusted Year 2021 base traffic volumes, are illustrated in **Figure 7**.

Year 2036 Total Projected Traffic Conditions

The development-generated traffic (Figure 6) was added to the Year 2036 no-build traffic volumes to determine the projected Year 2036 total projected traffic volumes, as shown in **Figure 8**.







5. Traffic Analysis and Recommendations

Capacity analyses were performed for the key intersections included in the study area to determine the ability of the existing roadway system to accommodate existing and future traffic demands. Analyses were performed for the weekday morning and weekday evening peak hours for the Year 2021 base, Year 2036 no build, and Year 2036 total projected conditions.

The traffic analyses were performed using the methodologies outlined in the Transportation Research Board's *Highway Capacity Manual (HCM)*, 6th Edition and analyzed using Synchro/SimTraffic 10 software. The analysis for the traffic-signal controlled intersections were accomplished using provided cycle lengths and phasings to determine the average overall vehicle delay and levels of service.

The analyses for the unsignalized intersections determine the average control delay to vehicles at an intersection. Control delay is the elapsed time from a vehicle joining the queue at a stop sign (includes the time required to decelerate to a stop) until its departure from the stop sign and resumption of free flow speed. The methodology analyzes each intersection approach controlled by a stop sign and considers traffic volumes on all approaches and lane characteristics.

The ability of an intersection to accommodate traffic flow is expressed in terms of level of service, which is assigned a letter from A to F based on the average control delay experienced by vehicles passing through the intersection. The *Highway Capacity Manual* definitions for levels of service and the corresponding control delay for signalized intersections and unsignalized intersections are included in the Appendix of this report.

Summaries of the traffic analysis results showing the level of service and overall intersection delay (measured in seconds) for the year 2021 base, Year 2036 no-build, and Year 2036 total projected conditions are presented in **Tables 2** through **11**. A discussion of each intersection follows. Summary sheets for the capacity analyses are included in the Appendix.

Table 2

CAPACITY ANALYSIS RESULTS – LINCOLN HIGHWAY WITH WOLF’S CROSSING ROAD

Peak Hour	Condition	Operating Conditions by Approach												
		Eastbound			Westbound			Northbound			Southbound			Overall
		L	T	R	L	T	R	L	T	R	L	T	R	
Weekday Morning	Year 2021 Base	F 80.5	C 33.8	C 34.1		B 10.5	D 50.2		B 19.8	C 31.4		D 46.7		
			C – 34.1			D – 48.1			C – 29.9					
	Year 2036 No Build	F 99+	D 44.1	D 38.9		B 13.0	F 99+		D 35.9	D 48.2		F 99+		
			D – 39.7			F – 99+			D – 46.6					
	Year 2036 Total Projected	F 99+	D 46.6	D 38.0		B 17.9	F 99+		D 46.6	E 67.9		F 99+		
			D – 39.2			F – 99+			E – 65.0					
Weekday Evening	Year 2021 Base	D 51.8	C 29.7	E 69.3		C 23.5	D 35.8		B 16.0	D 42.1		D 44.8		
			E – 64.1			C – 33.9			D – 38.5					
	Year 2036 No Build	F 99+	C 32.8	F 99+		D 41.6	D 54.9		D 36.8	E 72.0		F 95.3		
			F – 99+			D – 52.8			E – 67.2					
	Year 2036 Total Projected	F 99+	D 35.5	F 99+		E 75.1	F 99+		E 60.1	F 99+		F 99+		
			F – 99+			F – 99+			F – 99+					
Delay is Measured in Seconds L – Left, T – Through, R – Right														

Delay is Measured in Seconds L – Left, T – Through, R – Right

Table 3

CAPACITY ANALYSIS RESULTS – LINCOLN HIGHWAY WITH WOLF’S CROSSING ROAD
(ASSUMING POTENTIAL REGIONAL IMPROVEMENTS WITHIN THE AVAILABLE RIGHT-OF-WAY)

Peak Hour	Condition	Operating Conditions by Approach												Overall
		Eastbound			Westbound			Northbound			Southbound			
		L	T	R	L	T	R	L	T	R	L	T	R	
Weekday Morning	Year 2036 Total Projected	F 99+		A 7.3	D 36.3	C 34.3	B 14.0	C 25.1	F 99+		E 71.1	E 59.8		F 87.0
		F – 99+			C – 27.5			F – 99+			E – 61.3			
Weekday Evening	Year 2036 Total Projected	F 99+		A 9.6	D 42.2	D 52.9	B 17.6	F 87.8	E 71.3		E 71.4	F 98.4		E 76.1
		F – 92.1			D – 40.2			E – 73.9			F – 94.6			
Delay is Measured in Seconds L – Left, T – Through, R – Right														

Table 4

CAPACITY ANALYSIS RESULTS –WOLF’S CROSSING ROAD WITH EOLA ROAD

Peak Hour	Condition	Operating Conditions by Approach												
		Eastbound			Westbound			Northbound			Southbound			Overall
		L	T	R	L	T	R	L	T	R	L	T	R	
Weekday Morning	Year 2021 Base	B 19.5	C 28.6		B 16.1	D 36.9		A 9.2	B 15.0		A 9.3	B 17.4	B 17.9	C 20.8
		C – 25.8			D – 35.3			B – 13.5			B – 15.9			
	Year 2036 No Build	B 19.3	C 28.5		B 15.3	D 36.5		B 10.0	B 16.7		B 10.1	B 18.1	B 18.8	C 21.3
		C – 25.7			C – 34.9			B – 14.9			B – 16.7			
	Year 2036 Total Projected	B 18.2	C 34.6		B 16.3	C 33.1		B 11.0	B 14.4		B 11.1	B 19.7	A 4.9	B 19.8
		C – 29.8			C – 29.7			B – 13.6			B – 12.6			
Weekday Evening	Year 2021 Base	B 15.0	C 20.3		B 10.1	D 37.4		B 15.6	C 22.6		B 15.7	C 24.8	C 25.0	C 26.0
		B – 18.1			D – 35.4			C – 21.6			C – 23.7			
	Year 2036 No Build	B 16.0	B 18.8		A 9.7	D 41.7		B 16.5	C 24.4		B 16.6	C 27.5	C 27.4	C 28.2
		B – 17.8			D – 39.3			C – 23.2			C – 26.1			
	Year 2036 Total Projected	B 17.2	C 21.8		B 12.7	D 40.5		B 17.3	C 26.0		B 17.4	C 34.1	A 6.1	C 26.8
		C – 20.3			C – 34.4			C – 25.1			C – 24.0			
Delay is Measured in Seconds L – Left, T – Through, R – Right														

Delay is Measured in Seconds L – Left, T – Through, R – Right

Table 5

CAPACITY ANALYSIS RESULTS –LINCOLN HIGHWAY WITH EOLA ROAD

Peak Hour	Condition	Operating Conditions by Approach						
		Westbound		Northbound		Southbound		Overall
		L	R	T	R	L	T	
Weekday Morning	Year 2021 Base	D 45.1	--	B 10.0	A 5.4	A 3.0	A 7.5	B 11.1
		D – 45.1		B – 10.0		A – 7.5		
	Year 2036 No Build	D 45.3	--	B 16.0	A 6.2	A 4.0	B 11.8	B 14.8
		D – 45.3		B – 14.0		B – 11.8		
	Year 2036 Total Projected	D 46.7	B 13.3	D 41.5	A 2.6	A 6.6	B 16.2	C 27.2
		D – 42.1		C – 33.1		B – 15.8		
Weekday Evening	Year 2021 Base	D 51.8	C 29.0	B 14.5	A 7.4	A 4.5	B 13.3	B 17.4
		D – 51.7		B – 12.9		B – 13.2		
	Year 2036 No Build	E 57.6	C 29.0	B 17.7	A 7.8	A 4.5	B 16.6	C 20.7
		E – 57.5		B – 15.7		B – 16.6		
	Year 2036 Total Projected	F (F) 99+ (99+)	B (B) 18.0 (15.9)	F (E) 97.5 (67.9)	A (A) 3.5 (4.7)	C (E) 25.8 (64.4)	C (C) 24.2 (24.7)	E (E) 72.7 (58.5)
		F – 151.3 (F – 149.9)		E – 77.3 (E- 53.8)		C – 24.4 (C – 29.1)		
Delay is Measured in Seconds L – Left, T – Through, R – Right () – Assumes increasing the cycle length to 120 seconds								

Table 6

CAPACITY ANALYSIS RESULTS –LINCOLN HIGHWAY WITH 111th STREET

Peak Hour	Condition	Operating Conditions by Approach												
		Eastbound			Westbound			Northbound			Southbound			Overall
		L	T	R	L	T	R	L	T	R	L	T	R	
Weekday Morning	Year 2021 Base	F 80.5			C 33.8		C 34.1	B 10.5	D 50.2		B 19.8	C 31.4		D 46.7
					C – 34.1			D – 48.1			C – 29.9			
	Year 2036 No Build	E 78.8			D 37.9		D 52.8	A 4.3	E 68.7		E 69.5	A 9.1		D 54.6
					D – 48.2			E – 68.3			C – 31.0			
	Year 2036 Total Projected	F 96.6			D 53.6		A 9.3	A 4.5	F 98.7		F 99+	B 18.2		E 66.8
					C – 24.5			F – 98.2			D – 45.9			
Weekday Evening	Year 2021 Base	E 78.5			D 50.2		F 99+	A 4.4	B 18.4		A 7.2	C 25.7		D 37.9
					F 88.1			B – 18.1			D – 22.9			
	Year 2036 No Build	F 99+			E 57.0		F 99+	A 5.3	C 23.9		B 10.9	D 47.6		E 59.8
					F – 121.2			C – 23.4			D – 42.1			
	Year 2036 Total Projected	F 99+			F 99+		F 99+	A 5.3	E 56.7		F 99+	F 87.1		F 99+
					F – 99+			E – 55.6			F – 99+			

Table 7

CAPACITY ANALYSIS RESULTS –LINCOLN HIGHWAY WITH 111th STREET

(ASSUMING POTENTIAL REGIONAL IMPROVEMENTS WITHIN THE AVAILABLE RIGHT-OF-WAY)

Peak Hour	Condition	Operating Conditions by Approach												
		Eastbound			Westbound			Northbound			Southbound			Overall
		L	T	R	L	T	R	L	T	R	L	T	R	
Weekday Morning	Year 2036 Total Projected	D 41.0	E 65.2		D 41.4	D 49.8	B 18.2	A 6.7	F 99+		F 99+	B 16.8	A 1.3	E 65.6
		E – 56.5			C – 28.1			F – 99+			D – 42.4			
Weekday Evening	Year 2036 Total Projected	E 56.5	D 47.6		D 44.7	E 75.4	E 59.5	A 8.7	F 99+		F 89.4	D 39.9	A 1.2	E 68.2
		D – 52.4			E – 62.6			F – 99+			D – 44.6			

Table 8

CAPACITY ANALYSIS RESULTS –EOLA ROAD WITH ACCESS ROAD/COMMERCIAL DRIVE

Peak Hour	Condition	Operating Conditions by Approach												Overall
		Eastbound			Westbound			Northbound			Southbound			
		L	T	R	L	T	R	L	T	R	L	T	R	
Weekday Morning	Year 2036 Total Projected	C 26.7	B 18.9		C 31.0	B 13.6		A 7.5	B 11.3	A 0.0	A 5.2	A 8.9	A 0.0	B 12.4
		C – 21.7			C – 20.2			A – 9.9			A – 7.0			
Weekday Evening	Year 2036 Total Projected	C 32.4	B 12.9		C 26.3	B 17.1		A 9.7	B 18.7	A 1.7	A 6.6	B 12.3	A 0.6	B 14.9
		C – 24.4			C – 21.6			B – 14.0			A – 9.3			

Table 9
CAPACITY ANALYSIS RESULTS – UNSIGNALIZED INTERSECTIONS
YEAR 2021 BASE CONDITIONS

Intersection	Weekday Morning Peak Hour		Saturday Midday Peak Hour	
	LOS	Delay	LOS	Delay
Wolf's Crossing Road with Hoffman Boulevard				
• Eastbound Left Turn	A	7.6	A	8.6
• Southbound Approach	B	11.9	B	14.9
LOS = Level of Service Delay is measured in seconds.				

Table 10
CAPACITY ANALYSIS RESULTS – UNSIGNALIZED INTERSECTIONS
YEAR 2036 NO BUILD CONDITIONS

Intersection	Weekday Morning Peak Hour		Saturday Midday Peak Hour	
	LOS	Delay	LOS	Delay
Wolf's Crossing Road with Hoffman Boulevard				
• Eastbound Left Turn	A	7.6	A	8.8
• Southbound Approach	B	12.6	C	16.6
LOS = Level of Service Delay is measured in seconds.				

Table 11
CAPACITY ANALYSIS RESULTS – UNSIGNALIZED INTERSECTIONS
YEAR 2036 TOTAL PROJECTED CONDITIONS

Intersection	Weekday Morning Peak Hour		Saturday Midday Peak Hour	
	LOS	Delay	LOS	Delay
Wolf's Crossing Road with Hoffman Boulevard				
• Eastbound Left Turn	A	7.7	A	9.4
• Southbound Approach	B	14.4	C	21.9
Wolf's Crossing Road with the Lincoln Crossing Access Road				
• Westbound Left Turn	A	8.3	A	8.5
• Northbound Approach	B	13.4	C	19.4
Lincoln Highway with the Commercial Right-In/Right-Out Access Drive				
• Westbound Approach	C	19.5	D	27.5
Lincoln Highway with the Flex Residential/Del Webb Access Road				
• Westbound Left Turn	E	39.9	F	99+
• Westbound Right Turn	D	26.9	E	41.8
• Southbound Left Turn	B	12.2	C	17.6
111th Street with the Full Movement Access Road				
• Eastbound Left Turn	A	8.1	A	9.9
• Southbound Approach	C	15.3	D	33.1
LOS = Level of Service Delay is measured in seconds.				

6. Discussion and Recommendations

The following is an evaluation of the analyzed intersections based on the projected traffic volumes and the capacity analyses performed.

Lincoln Highway and Wolf's Crossing Road

Based on the results of the capacity analyses, this intersection is operating at an overall acceptable LOS D during the weekday morning and evening peak hours. However, it is important to note that various movements and approaches are operating at/or above capacity. Under Year 2036 No Build, this intersection will operate at a LOS F, given the anticipated growth in the area and will continue to do so under Year 2036 Future Conditions. In order to mitigate these deficiencies, which are primarily due to existing conditions and the background traffic conditions, the intersection will require major modifications as part of a regional improvement plan for the area. These were identified in the Village of Oswego Pre-Phase I study of the Wolf's Crossing Road corridor. Given that the identified improvements will require significant right-of-way acquisition and that there are no funds or timeframe allocated for these improvements, KLOA, Inc. has identified interim regional improvements that could potentially be accommodated within the available right-of-way. These improvements are as follows:

- Provide an exclusive westbound to northbound right-turn lane
- Provide an exclusive eastbound to southbound right-turn lane
- Provide right-turn overlap phases for the eastbound and westbound right-turn lanes
- Increasing the cycle length to 120 seconds

Assuming these regional improvements, the intersection overall delay will be reduced by approximately 40 to 50 percent and the eastbound and westbound queues will be reduced substantially, thus having a positive impact on the east-west traffic flow. As previously indicated, these improvements have been identified as part of the Village of Oswego Pre-Phase I study, are needed under Existing and No Build Conditions and are not solely due to the proposed development. An exhibit depicting these improvements is enclosed in the Appendix.

Wolf's Crossing Road and Eola Road

This intersection operates at an overall acceptable LOS during the weekday morning and evening peak hour with all four approaches operating at a LOS D or better. Under Year 2036 No Build conditions and the Year 2036 Future conditions, the intersection will continue to operate at an overall acceptable LOS thus indicating the intersection has sufficient capacity to accommodate the estimated background growth and the site traffic volumes. As such, no additional geometric or signal timing improvements are necessary at this intersection in conjunction with the proposed development.

Lincoln Highway and Eola Road

As previously indicated, the Illinois Department of Transportation (IDOT) recently improved this intersection. The improvement involved realigning Eola Road to intersect Lincoln Highway at a perpendicular angle and signalizing the intersection with a cycle length of 90 seconds per the prepared IDS. In addition, exclusive left and right-turn lanes were provided on Lincoln Highway and Eola Road was widened to provide an exclusive left-turn lane and an exclusive right-turn lane. Based on the results of the capacity analyses, the intersection is operating at an overall acceptable LOS. Under Year 2036 No Build traffic conditions, the intersection will continue to operate at an overall acceptable LOS.

Under Year 2036 Future conditions and in order to accommodate the future traffic volumes, the traffic signal cycle length during the weekday evening peak period should be increased to 120 seconds. No additional improvements are necessary in conjunction with the proposed development.

Lincoln Highway and 111th Street

Based on the results of the capacity analyses, this intersection is operating at an overall acceptable LOS D during the weekday morning and evening peak hours. However, it is important to note that various movements and approaches are operating at/or above capacity. Under Year 2036 No Build conditions, the intersection will operate at a LOS E given the anticipated growth in the area and various movements will operate at a LOS E or F. Based on a review of the No Build traffic volumes, the intersection will require regional improvements to mitigate these deficiencies such as widening Lincoln Highway to provide two through lanes in each direction and the widening of the eastbound and westbound approaches of 111th Street to provide exclusive left-turn lanes. While under Year 2036 Future conditions, the intersection will experience longer delays, this is primarily due to existing deficiencies and the background growth and as previously indicated should be mitigated as part of regional improvements developed for the Lincoln Highway corridor.

Given that there is no right-of-way available along Lincoln Highway to provide two lanes in each direction, KLOA, Inc. has identified interim regional improvements that could be accommodated within the available right-of-way. These improvements are as follows:

- Provide an exclusive westbound to southbound left-turn lane
- Provide an exclusive eastbound to northbound left-turn lane
- Provide right-turn overlap phase for the existing westbound right-turn lane
- Provide an exclusive southbound to westbound right-turn lane
- Provide right-turn overlap for the future southbound right-turn lane
- Increase the cycle length to 120 seconds

Assuming these regional improvements, the intersection overall delay will be reduced by approximately 45 percent and the eastbound and westbound queues will be reduced substantially thus having a positive impact on the east-west traffic flow. As previously indicated, these improvements are regional in nature and not solely due to the proposed development. An exhibit depicting these improvements is included in the Appendix.

Eola Road with Access Road/Commercial Full Access Drive

Eola Access Road will serve as a main entrance to the Lincoln Prairie by Del Webb and a secondary entrance for Lincoln Crossing. No widening is required to accommodate these residential developments or the development of Flex Parcel 1. As such, in the interim, Eola Road will provide a shared through/right-turn lane in the northbound direction and the southbound direction will provide an exclusive left-turn lane and a through lane. Upon development of Commercial Parcel 1, Eola Road should be widened to provide two lanes in each direction. In addition, an exclusive northbound left-turn lane and an exclusive southbound right-turn lane as well as an exclusive northbound right-turn lane will be provided on Eola Road. Given the wide cross-section of Eola Road and depending on ultimate scope of the development on Commercial Parcel 1, a traffic signal should be considered. Based on the results of the capacity analyses, this intersection, assuming full buildout, will operate at an overall acceptable LOS and as such no additional geometric or traffic control improvements will be necessary in conjunction with the proposed development.

Wolf's Crossing Road with Hoffman Boulevard

Based on the results of the capacity analyses, all of the turning movements at this intersection are operating at an acceptable LOS. Under Year 2036 Future conditions, all of the turning movements will continue to operate at an acceptable LOS. As such, no geometric or traffic control improvements are necessary at this intersection in conjunction with the development.

Wolf's Crossing Road and Lincoln Crossing Full Access Road

This intersection will serve the Pulte Homes subdivision. Based on the projected traffic volumes and for design consistency, Wolf's Crossing Road should be restriped to provide for an exclusive westbound left-turn lane. A review of IDOT's turn lane warrants and the projected traffic volumes indicate that an exclusive right-turn lane into the site will not be necessary. Based on the results of the capacity analyses, all of the turning movements at this intersection will operate at an overall acceptable LOS. As such, no additional improvements will be necessary in conjunction with the proposed development.

Lincoln Highway and Commercial Right-in/Right-Out

This restricted access drive will provide additional accessibility to the northern commercial parcel and will be located approximately 820 feet north of the Lincoln Highway signalized intersection with Eola Road. Based on the projected traffic volumes, an exclusive right-turn lane into the site will be provided. Inspection of the capacity analyses indicate that the outbound movement from this access drive will operate at an acceptable LOS during both peak hours. As such, no additional geometric or traffic control improvements will be necessary in conjunction with the proposed development.

Lincoln Highway and Flex Residential/Del Webb Access Road

This access road will serve the Flex Residential parcels and will provide additional accessibility to Lincoln Prairie by Del Webb. Given the projected traffic volumes, Lincoln Highway will be widened to provide for a southbound exclusive left-turn lane and a northbound exclusive right-turn lane. Based on the results of the capacity analyses, all of the turning movements will operate at an acceptable LOS with the exception of the outbound left-turn movement which will operate at a LOS E and F during the weekday morning and evening peak hours, respectively. This is not uncommon, and it is expected at an unsignalized intersection where a minor road intersects a major arterial like Lincoln Highway. As such, no additional geometric or traffic control improvements will be necessary in conjunction with the proposed development.

111th Street and Full Movement Access Road

This intersection will serve the Flex Residential parcels and Commercial Parcel 2. Based on the projected traffic volumes, 111th Street should be widened to provide for an exclusive eastbound left-turn lane. Based on the results of the capacity analyses, all of the turning movements at this intersection will operate at an overall acceptable LOS. No additional improvements will be necessary in conjunction with the proposed development.

7. Conclusion

A traffic impact study was conducted for the proposed Lincoln Prairie, a mixed-use development to be located on the eastside of Lincoln Highway (US Route 30) between Wolf's Crossing and 111th Street in Aurora, Illinois. The site, which is currently utilized for agricultural purposes, is generally bounded by Wolf's Crossing Road to the north, Lincoln Highway to the west, 111th Street to the south and an E.J.&E Railroad to the east. The plans call for single-family homes, a Del Webb Active Adult Community, and approximately 39 acres of commercial development.

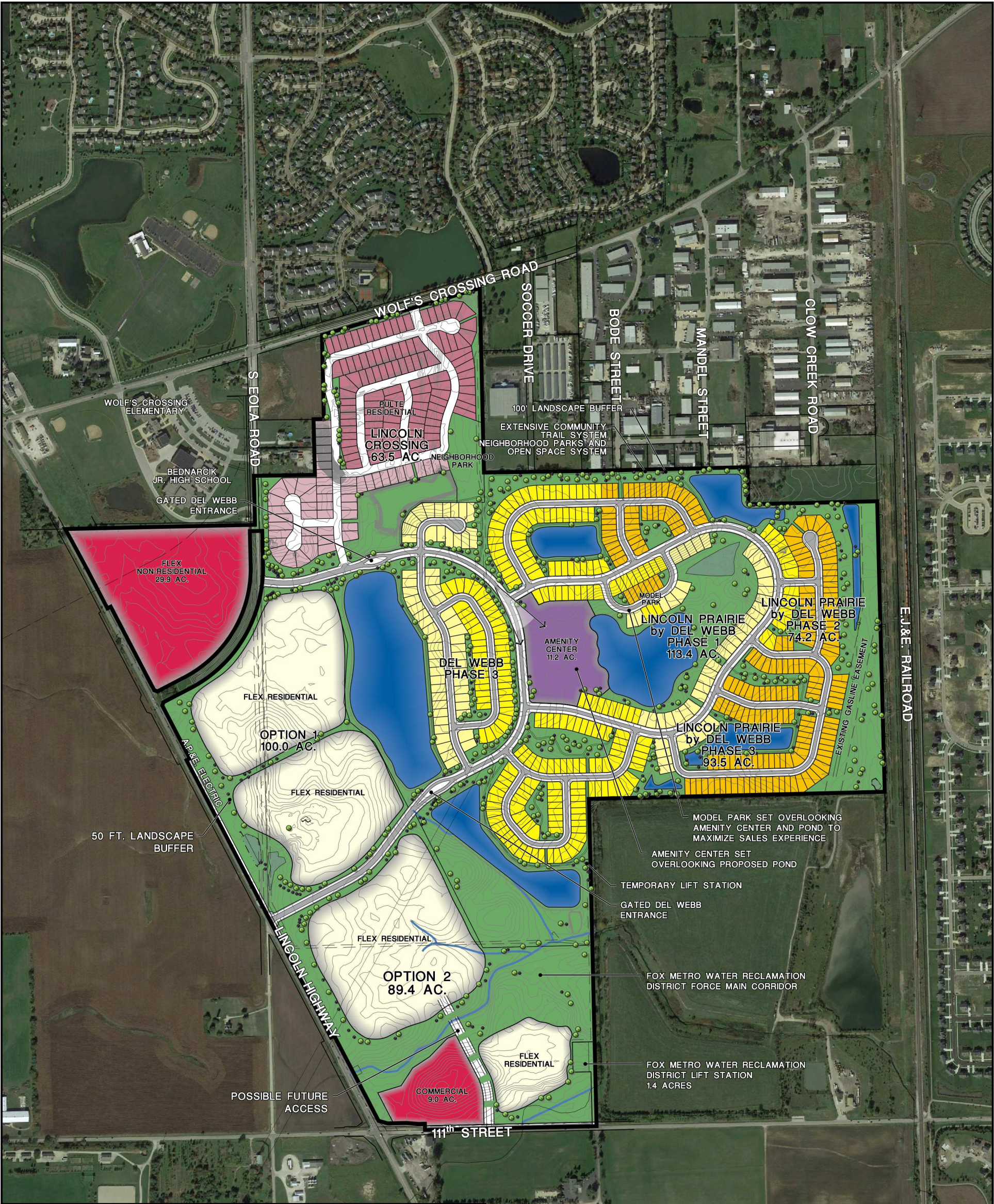
Based on the proposed development plan and the traffic capacity analyses for the full buildout of the development, the findings and recommendations of this study are outlined below:

- The proposed development-generated traffic will be consistent and compatible with traffic patterns and volumes in the area.
- Overall, the access system with recommended improvements/modifications will be adequate to accommodate the projected traffic estimated to be generated by the proposed redevelopment.
- The mixture of residential and retail/commercial land uses will result in internal capture of trips resulting in a reduction in the overall trips that will be generated by the proposed development.
- Based on a review of the existing traffic volumes and the Year 2036 No Build projections, capacity deficiencies will continue to exist at the intersections of Lincoln Highway with Wolf's Crossing Road and 111th Street that should be mitigated as part of interim regional improvements and not solely due to site generated traffic volumes.
- Exclusive left-turn lanes should be provided on Wolf's Crossing Road and 111th Street at their respective intersections with the proposed access roads.
- Lincoln Highway at its intersection with the proposed Flex Residential/Del Webb access road should be widened to provide for an exclusive left-turn lane and an exclusive right-turn lane.
- An exclusive right-turn lane should be provided on Lincoln Highway at its intersection with the proposed commercial right-in/right-out access drive.
- Eola Road existing cross-section (3 lanes) will be sufficient to accommodate the traffic that will be generated by Lincoln Crossing and Lincoln Prairie by Del Webb.
- With full buildout of the development including the commercial parcels, Eola Road should be widened to 5 lanes and a traffic signal should be installed at its intersection with the access road to Del Webb and the access drive to the commercial parcel.

Appendix

Site Plan
Traffic Count Summary Sheets
CMAP Traffic Projection Letter/ITE Data
Level of Service Criteria
Capacity Analysis – Existing Conditions
Capacity Analysis – Year 2036 No Build Conditions
Capacity Analysis – Year 2036 Total Conditions
Preliminary Regional Geometric Improvements

Site Plan



Lot Summary

Del Webb

Product Type	Typ. Lot Size	Phase 1	Phase 2	Phase 3	Total	Mix
Scenic	44' x 120'	52 units	69 units	81 units	202 units	36.73%
Distinctive	50' x 120'	77 units	62 units	103 units	242 units	44.00%
Echelon	64' x 120'	27 units	36 units	43 units	106 units	19.27%
		156 units	167 units	227 units	550 units	

Pulte

Product Type	Typ. Lot Size	Phase 1	Phase 2	Phase 3	Total	Mix
Meadows	55' x 130'	111 units			111 units	68.10%
Estates	62' x 130'	52 units			52 units	31.90%
		163 units			163 units	

Community Total:

713 units

COMPOSITE LOTTING PLAN
LINCOLN PRAIRIE

PulteGroup, Inc.
AURORA, ILLINOIS



0 400 800 1200

Scale: 1" = 800'
Date: March 17, 2021

Traffic Count Summary Sheets



Kenig Lindgren O'Hara Aboona, Inc.
9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018
(847)518-9990

Count Name: Lincoln Highway with Wolf's
Crossing Road
Site Code:
Start Date: 01/07/2021
Page No: 1

Turning Movement Data

Start Time	Wolf's Crossing Road Eastbound						Wolf's Crossing Road Westbound						Lincoln Highway Northbound						Lincoln Highway Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:00 AM	0	2	39	13	0	54	0	2	20	9	0	31	0	3	87	2	0	92	0	13	84	3	0	100	277
7:15 AM	0	5	50	25	0	80	0	7	28	14	0	49	0	8	96	2	0	106	0	14	91	2	0	107	342
7:30 AM	0	10	57	21	0	88	0	6	34	11	0	51	0	11	115	1	0	127	0	25	118	3	0	146	412
7:45 AM	0	2	70	14	0	86	0	9	35	18	0	62	0	7	106	1	0	114	0	33	102	0	0	135	397
Hourly Total	0	19	216	73	0	308	0	24	117	52	0	193	0	29	404	6	0	439	0	85	395	8	0	488	1428
8:00 AM	0	4	41	24	0	69	0	8	29	7	0	44	0	9	93	4	0	106	0	17	98	1	0	116	335
8:15 AM	0	6	46	24	0	76	0	9	36	10	0	55	0	15	93	1	0	109	0	22	86	2	0	110	350
8:30 AM	0	3	66	13	0	82	0	10	29	17	0	56	0	15	89	2	0	106	0	17	92	2	0	111	355
8:45 AM	0	1	59	17	0	77	0	5	33	18	0	56	0	12	105	6	0	123	0	18	74	2	0	94	350
Hourly Total	0	14	212	78	0	304	0	32	127	52	0	211	0	51	380	13	0	444	0	74	350	7	0	431	1390
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4:00 PM	0	1	61	19	0	81	0	8	91	23	0	122	0	23	152	8	0	183	0	33	115	4	0	152	538
4:15 PM	0	2	53	14	0	69	0	4	86	23	0	113	0	27	139	4	0	170	0	19	117	8	0	144	496
4:30 PM	0	6	57	10	0	73	0	9	85	34	0	128	0	20	127	3	0	150	0	30	120	4	0	154	505
4:45 PM	0	2	52	20	0	74	0	4	61	34	0	99	0	32	148	3	0	183	0	30	132	6	0	168	524
Hourly Total	0	11	223	63	0	297	0	25	323	114	0	462	0	102	566	18	0	686	0	112	484	22	0	618	2063
5:00 PM	0	1	45	17	0	63	0	6	66	30	0	102	0	25	91	2	0	118	0	26	123	3	0	152	435
5:15 PM	0	3	59	19	0	81	0	6	87	25	0	118	0	23	150	4	0	177	0	27	130	4	0	161	537
5:30 PM	0	4	59	15	0	78	0	7	75	27	0	109	0	23	109	3	0	135	0	29	130	2	0	161	483
5:45 PM	0	0	52	18	0	70	0	9	75	37	0	121	0	24	144	2	0	170	0	23	112	4	0	139	500
Hourly Total	0	8	215	69	0	292	0	28	303	119	0	450	0	95	494	11	0	600	0	105	495	13	0	613	1955
Grand Total	0	52	866	283	0	1201	0	109	870	337	0	1316	0	277	1844	48	0	2169	0	376	1724	50	0	2150	6836
Approach %	0.0	4.3	72.1	23.6	-	-	0.0	8.3	66.1	25.6	-	-	0.0	12.8	85.0	2.2	-	-	0.0	17.5	80.2	2.3	-	-	-
Total %	0.0	0.8	12.7	4.1	-	17.6	0.0	1.6	12.7	4.9	-	19.3	0.0	4.1	27.0	0.7	-	31.7	0.0	5.5	25.2	0.7	-	31.5	-
Lights	0	51	855	282	-	1188	0	105	862	323	-	1290	0	272	1692	46	-	2010	0	358	1578	48	-	1984	6472
% Lights	-	98.1	98.7	99.6	-	98.9	-	96.3	99.1	95.8	-	98.0	-	98.2	91.8	95.8	-	92.7	-	95.2	91.5	96.0	-	92.3	94.7
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Single-Unit Trucks	0	1	11	1	-	13	0	2	7	7	-	16	0	5	51	1	-	57	0	13	62	2	-	77	163
% Single-Unit Trucks	-	1.9	1.3	0.4	-	1.1	-	1.8	0.8	2.1	-	1.2	-	1.8	2.8	2.1	-	2.6	-	3.5	3.6	4.0	-	3.6	2.4
Articulated Trucks	0	0	0	0	-	0	0	2	1	7	-	10	0	0	101	1	-	102	0	5	84	0	-	89	201
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	1.8	0.1	2.1	-	0.8	-	0.0	5.5	2.1	-	4.7	-	1.3	4.9	0.0	-	4.1	2.9
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0

[illegible]

[illegible]

Kenig Lindgren O'Hara Aboona, Inc.
9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018
(847)518-9990

Count Name: Eola Road with Lincoln Highway
Site Code:
Start Date: 01/07/2021
Page No: 1

Turning Movement Data

Start Time	Eola Road Westbound					Lincoln Highway Northbound					Lincoln Highway Southbound					Int. Total
	U-Turn	Left	Right	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	
7:00 AM	0	25	0	0	25	0	92	33	0	125	0	0	94	0	94	244
7:15 AM	0	40	0	0	40	0	106	45	0	151	0	0	120	0	120	311
7:30 AM	0	26	0	0	26	0	130	67	0	197	0	1	136	0	137	360
7:45 AM	0	25	0	0	25	0	117	58	0	175	0	0	142	0	142	342
Hourly Total	0	116	0	0	116	0	445	203	0	648	0	1	492	0	493	1257
8:00 AM	0	34	0	0	34	0	106	39	0	145	0	0	127	0	127	306
8:15 AM	0	23	0	0	23	0	106	43	0	149	0	0	125	0	125	297
8:30 AM	0	33	0	0	33	0	106	45	0	151	0	0	103	0	103	287
8:45 AM	0	24	0	0	24	0	124	48	0	172	0	0	112	0	112	308
Hourly Total	0	114	0	0	114	0	442	175	0	617	0	0	467	0	467	1198
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4:00 PM	0	53	0	0	53	0	158	32	0	190	0	0	135	0	135	378
4:15 PM	0	56	1	0	57	0	176	56	0	232	0	1	134	0	135	424
4:30 PM	0	69	1	0	70	0	161	48	0	209	0	0	147	0	147	426
4:45 PM	0	60	0	0	60	0	176	57	0	233	0	0	160	0	160	453
Hourly Total	0	238	2	0	240	0	671	193	0	864	0	1	576	0	577	1681
5:00 PM	0	55	0	0	55	0	117	52	0	169	0	0	126	0	126	350
5:15 PM	0	48	0	0	48	0	175	61	0	236	0	2	158	0	160	444
5:30 PM	0	63	0	0	63	0	139	34	0	173	0	0	152	0	152	388
5:45 PM	0	52	0	0	52	0	160	49	0	209	0	0	138	0	138	399
Hourly Total	0	218	0	0	218	0	591	196	0	787	0	2	574	0	576	1581
Grand Total	0	686	2	0	688	0	2149	767	0	2916	0	4	2109	0	2113	5717
Approach %	0.0	99.7	0.3	-	-	0.0	73.7	26.3	-	-	0.0	0.2	99.8	-	-	-
Total %	0.0	12.0	0.0	-	12.0	0.0	37.6	13.4	-	51.0	0.0	0.1	36.9	-	37.0	-
Lights	0	675	2	-	677	0	1993	751	-	2744	0	3	1963	-	1966	5387
% Lights	-	98.4	100.0	-	98.4	-	92.7	97.9	-	94.1	-	75.0	93.1	-	93.0	94.2
Buses	0	1	0	-	1	0	0	0	-	0	0	0	0	-	0	1
% Buses	-	0.1	0.0	-	0.1	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Single-Unit Trucks	0	6	0	-	6	0	50	10	-	60	0	0	61	-	61	127
% Single-Unit Trucks	-	0.9	0.0	-	0.9	-	2.3	1.3	-	2.1	-	0.0	2.9	-	2.9	2.2
Articulated Trucks	0	4	0	-	4	0	106	6	-	112	0	1	85	-	86	202
% Articulated Trucks	-	0.6	0.0	-	0.6	-	4.9	0.8	-	3.8	-	25.0	4.0	-	4.1	3.5
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

[illegible]

Turning Movement Peak Hour Data (1997-1998)																
Start Time	Eola Road Westbound					Lincoln Highway Northbound					Lincoln Highway Southbound					Int. Total
	U-Turn	Left	Right	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	
4:30 PM	0	69	1	0	70	0	161	48	0	209	0	0	147	0	147	426
4:45 PM	0	60	0	0	60	0	176	57	0	233	0	0	160	0	160	453
5:00 PM	0	55	0	0	55	0	117	52	0	169	0	0	126	0	126	350
5:15 PM	0	48	0	0	48	0	175	61	0	236	0	2	158	0	160	444
Total	0	232	1	0	233	0	629	218	0	847	0	2	591	0	593	1673
Approach %	0.0	99.6	0.4	-	-	0.0	74.3	25.7	-	-	0.0	0.3	99.7	-	-	-
Total %	0.0	13.9	0.1	-	13.9	0.0	37.6	13.0	-	50.6	0.0	0.1	35.3	-	35.4	-
PHF	0.000	0.841	0.250	-	0.832	0.000	0.893	0.893	-	0.897	0.000	0.250	0.923	-	0.927	0.923
Lights	0	229	1	-	230	0	597	215	-	812	0	2	562	-	564	1606
% Lights	-	98.7	100.0	-	98.7	-	94.9	98.6	-	95.9	-	100.0	95.1	-	95.1	96.0
Buses	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Buses	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Single-Unit Trucks	0	1	0	-	1	0	8	2	-	10	0	0	14	-	14	25
% Single-Unit Trucks	-	0.4	0.0	-	0.4	-	1.3	0.9	-	1.2	-	0.0	2.4	-	2.4	1.5
Articulated Trucks	0	2	0	-	2	0	24	1	-	25	0	0	15	-	15	42
% Articulated Trucks	-	0.9	0.0	-	0.9	-	3.8	0.5	-	3.0	-	0.0	2.5	-	2.5	2.5
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-



Kenig Lindgren O'Hara Aboona, Inc.
9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018
(847)518-9990

Count Name: Lincoln Highway with 111th Street
Site Code:
Start Date: 01/07/2021
Page No: 1

Turning Movement Data

Start Time	111th Street Eastbound						111th Street Westbound						Lincoln highway Northbound						Lincoln Highway Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:00 AM	0	21	20	1	0	42	0	2	13	27	0	42	0	0	95	6	0	101	0	40	74	9	0	123	308
7:15 AM	0	26	28	1	0	55	0	2	13	35	0	50	0	1	91	2	0	94	0	60	85	13	0	158	357
7:30 AM	0	26	39	1	0	66	0	0	21	34	0	55	0	1	115	7	0	123	0	61	84	12	0	157	401
7:45 AM	0	15	34	3	0	52	0	1	10	45	0	56	0	1	124	5	0	130	0	70	87	9	0	166	404
Hourly Total	0	88	121	6	0	215	0	5	57	141	0	203	0	3	425	20	0	448	0	231	330	43	0	604	1470
8:00 AM	0	17	25	2	0	44	0	3	23	36	0	62	0	0	95	4	0	99	0	52	94	11	0	157	362
8:15 AM	0	17	34	3	0	54	0	1	8	37	0	46	0	3	94	2	0	99	0	44	90	12	0	146	345
8:30 AM	0	17	30	0	0	47	0	0	9	45	0	54	0	0	113	2	0	115	0	43	83	15	0	141	357
8:45 AM	0	27	18	2	0	47	0	2	11	29	0	42	0	1	87	7	0	95	0	35	80	8	0	123	307
Hourly Total	0	78	107	7	0	192	0	6	51	147	0	204	0	4	389	15	0	408	0	174	347	46	0	567	1371
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4:00 PM	0	9	24	4	0	37	0	2	53	70	0	125	0	6	104	4	0	114	0	43	118	23	0	184	460
4:15 PM	0	15	25	1	0	41	0	4	59	75	0	138	0	3	139	3	0	145	0	46	107	39	0	192	516
4:30 PM	0	27	18	3	0	48	0	6	46	77	0	129	0	5	118	3	0	126	0	33	146	24	0	203	506
4:45 PM	0	15	18	2	0	35	0	3	46	68	0	117	0	4	137	3	0	144	0	47	147	28	0	222	518
Hourly Total	0	66	85	10	0	161	0	15	204	290	0	509	0	18	498	13	0	529	0	169	518	114	0	801	2000
5:00 PM	0	11	10	0	0	21	0	4	42	57	0	103	0	5	101	2	0	108	0	36	122	26	0	184	416
5:15 PM	0	18	16	1	0	35	0	3	53	74	0	130	0	3	127	1	0	131	0	46	138	25	0	209	505
5:30 PM	0	9	11	1	0	21	0	0	35	62	0	97	0	3	110	3	0	116	0	49	146	28	0	223	457
5:45 PM	0	22	20	0	0	42	0	3	24	74	0	101	0	4	124	3	0	131	0	43	120	25	0	188	462
Hourly Total	0	60	57	2	0	119	0	10	154	267	0	431	0	15	462	9	0	486	0	174	526	104	0	804	1840
Grand Total	0	292	370	25	0	687	0	36	466	845	0	1347	0	40	1774	57	0	1871	0	748	1721	307	0	2776	6681
Approach %	0.0	42.5	53.9	3.6	-	-	0.0	2.7	34.6	62.7	-	-	0.0	2.1	94.8	3.0	-	-	0.0	26.9	62.0	11.1	-	-	-
Total %	0.0	4.4	5.5	0.4	-	10.3	0.0	0.5	7.0	12.6	-	20.2	0.0	0.6	26.6	0.9	-	28.0	0.0	11.2	25.8	4.6	-	41.6	-
Lights	0	291	366	23	-	680	0	28	462	809	-	1299	0	39	1641	51	-	1731	0	721	1597	304	-	2622	6332
% Lights	-	99.7	98.9	92.0	-	99.0	-	77.8	99.1	95.7	-	96.4	-	97.5	92.5	89.5	-	92.5	-	96.4	92.8	99.0	-	94.5	94.8
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	1	0	-	1	0	2	0	0	-	2	3
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.1	0.0	-	0.1	-	0.3	0.0	0.0	-	0.1	0.0
Single-Unit Trucks	0	1	4	2	-	7	0	1	4	16	-	21	0	1	40	2	-	43	0	16	44	3	-	63	134
% Single-Unit Trucks	-	0.3	1.1	8.0	-	1.0	-	2.8	0.9	1.9	-	1.6	-	2.5	2.3	3.5	-	2.3	-	2.1	2.6	1.0	-	2.3	2.0
Articulated Trucks	0	0	0	0	-	0	0	7	0	20	-	27	0	0	92	4	-	96	0	9	80	0	-	89	212
% Articulated Trucks	-	0.0	0.0	0.0	-	0.0	-	19.4	0.0	2.4	-	2.0	-	0.0	5.2	7.0	-	5.1	-	1.2	4.6	0.0	-	3.2	3.2
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0

[illegible]

[illegible]



Kenig Lindgren O'Hara Aboona, Inc.
9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018
(847)518-9990

Count Name: Wolf's Crossing Road with Eola Road
Site Code:
Start Date: 01/07/2021
Page No: 1

Turning Movement Data

Start Time	Wolf's Crossing Road Eastbound						Wolf's Crossing Road Westbound						Eola Road Northbound						Eola Road Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
7:00 AM	0	21	36	0	0	57	0	1	20	8	0	29	0	1	25	6	0	32	0	10	30	11	0	51	169
7:15 AM	0	27	37	0	0	64	0	1	29	4	0	34	0	0	40	1	0	41	0	9	34	21	0	64	203
7:30 AM	0	21	55	0	0	76	0	1	25	5	0	31	0	0	60	10	0	70	0	14	31	19	0	64	241
7:45 AM	0	26	77	0	0	103	0	3	23	3	0	29	0	0	45	7	0	52	0	14	26	24	0	64	248
Hourly Total	0	95	205	0	0	300	0	6	97	20	0	123	0	1	170	24	0	195	0	47	121	75	0	243	861
8:00 AM	0	26	37	0	0	63	0	3	22	5	0	30	0	0	34	4	0	38	0	15	32	14	0	61	192
8:15 AM	0	19	44	0	0	63	0	6	27	4	0	37	0	0	35	8	0	43	0	14	16	18	0	48	191
8:30 AM	0	24	52	0	0	76	0	5	25	6	0	36	0	0	35	9	0	44	0	12	29	17	0	58	214
8:45 AM	0	27	55	0	0	82	0	2	36	11	0	49	0	1	30	14	0	45	0	12	20	15	0	47	223
Hourly Total	0	96	188	0	0	284	0	16	110	26	0	152	0	1	134	35	0	170	0	53	97	64	0	214	820
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4:00 PM	0	32	62	0	0	94	0	9	76	10	0	95	0	0	29	4	0	33	0	15	38	28	0	81	303
4:15 PM	0	23	51	1	0	75	0	14	80	25	0	119	0	1	44	6	0	51	0	12	50	34	0	96	341
4:30 PM	0	32	43	0	0	75	0	9	78	17	0	104	0	0	43	11	0	54	0	12	54	38	2	104	337
4:45 PM	0	29	51	1	0	81	0	7	55	19	0	81	0	1	46	4	0	51	0	9	52	32	0	93	306
Hourly Total	0	116	207	2	0	325	0	39	289	71	0	399	0	2	162	25	0	189	0	48	194	132	2	374	1287
5:00 PM	0	24	46	0	0	70	0	10	69	19	0	98	0	1	46	5	0	52	0	17	50	34	0	101	321
5:15 PM	0	27	61	0	0	88	0	10	56	27	0	93	0	0	56	4	0	60	0	12	51	39	0	102	343
5:30 PM	0	27	53	2	0	82	0	9	76	23	0	108	0	0	31	3	0	34	0	10	46	44	0	100	324
5:45 PM	0	40	33	0	0	73	0	7	62	19	0	88	0	0	44	6	0	50	0	11	48	46	0	105	316
Hourly Total	0	118	193	2	0	313	0	36	263	88	0	387	0	1	177	18	0	196	0	50	195	163	0	408	1304
Grand Total	0	425	793	4	0	1222	0	97	759	205	0	1061	0	5	643	102	0	750	0	198	607	434	2	1239	4272
Approach %	0.0	34.8	64.9	0.3	-	-	0.0	9.1	71.5	19.3	-	-	0.0	0.7	85.7	13.6	-	-	0.0	16.0	49.0	35.0	-	-	-
Total %	0.0	9.9	18.6	0.1	-	28.6	0.0	2.3	17.8	4.8	-	24.8	0.0	0.1	15.1	2.4	-	17.6	0.0	4.6	14.2	10.2	-	29.0	-
Lights	0	422	767	4	-	1193	0	91	740	202	-	1033	0	5	629	96	-	730	0	194	600	430	-	1224	4180
% Lights	-	99.3	96.7	100.0	-	97.6	-	93.8	97.5	98.5	-	97.4	-	100.0	97.8	94.1	-	97.3	-	98.0	98.8	99.1	-	98.8	97.8
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	1	0	-	1	1
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.2	0.0	-	0.1	0.0
Single-Unit Trucks	0	3	21	0	-	24	0	4	14	1	-	19	0	0	9	3	-	12	0	3	4	3	-	10	65
% Single-Unit Trucks	-	0.7	2.6	0.0	-	2.0	-	4.1	1.8	0.5	-	1.8	-	0.0	1.4	2.9	-	1.6	-	1.5	0.7	0.7	-	0.8	1.5
Articulated Trucks	0	0	5	0	-	5	0	2	5	2	-	9	0	0	5	3	-	8	0	1	2	1	-	4	26
% Articulated Trucks	-	0.0	0.6	0.0	-	0.4	-	2.1	0.7	1.0	-	0.8	-	0.0	0.8	2.9	-	1.1	-	0.5	0.3	0.2	-	0.3	0.6
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0

[illegible]



Kenig Lindgren O'Hara Aboona, Inc.
9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018
(847)518-9990

Count Name: Wolf's Crossing Road with Eola Road
Site Code:
Start Date: 01/07/2021
Page No: 4

Turning Movement Peak Hour Data (4:30 PM)

Start Time	Wolf's Crossing Road Eastbound						Wolf's Crossing Road Westbound						Eola Road Northbound						Eola Road Southbound						Int. Total
	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	
4:30 PM	0	32	43	0	0	75	0	9	78	17	0	104	0	0	43	11	0	54	0	12	54	38	2	104	337
4:45 PM	0	29	51	1	0	81	0	7	55	19	0	81	0	1	46	4	0	51	0	9	52	32	0	93	306
5:00 PM	0	24	46	0	0	70	0	10	69	19	0	98	0	1	46	5	0	52	0	17	50	34	0	101	321
5:15 PM	0	27	61	0	0	88	0	10	56	27	0	93	0	0	56	4	0	60	0	12	51	39	0	102	343
Total	0	112	201	1	0	314	0	36	258	82	0	376	0	2	191	24	0	217	0	50	207	143	2	400	1307
Approach %	0.0	35.7	64.0	0.3	-	-	0.0	9.6	68.6	21.8	-	-	0.0	0.9	88.0	11.1	-	-	0.0	12.5	51.8	35.8	-	-	-
Total %	0.0	8.6	15.4	0.1	-	24.0	0.0	2.8	19.7	6.3	-	28.8	0.0	0.2	14.6	1.8	-	16.6	0.0	3.8	15.8	10.9	-	30.6	-
PHF	0.000	0.875	0.824	0.250	-	0.892	0.000	0.900	0.827	0.759	-	0.904	0.000	0.500	0.853	0.545	-	0.904	0.000	0.735	0.958	0.917	-	0.962	0.953
Lights	0	112	197	1	-	310	0	32	251	82	-	365	0	2	189	21	-	212	0	49	205	142	-	396	1283
% Lights	-	100.0	98.0	100.0	-	98.7	-	88.9	97.3	100.0	-	97.1	-	100.0	99.0	87.5	-	97.7	-	98.0	99.0	99.3	-	99.0	98.2
Buses	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Buses	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Single-Unit Trucks	0	0	3	0	-	3	0	3	6	0	-	9	0	0	2	2	-	4	0	1	1	1	-	3	19
% Single-Unit Trucks	-	0.0	1.5	0.0	-	1.0	-	8.3	2.3	0.0	-	2.4	-	0.0	1.0	8.3	-	1.8	-	2.0	0.5	0.7	-	0.8	1.5
Articulated Trucks	0	0	1	0	-	1	0	1	1	0	-	2	0	0	0	1	-	1	0	0	1	0	-	1	5
% Articulated Trucks	-	0.0	0.5	0.0	-	0.3	-	2.8	0.4	0.0	-	0.5	-	0.0	0.0	4.2	-	0.5	-	0.0	0.5	0.0	-	0.3	0.4
Bicycles on Road	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	2	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	100.0	-	-

Turning Movement Data

Start Time	Wolf's Crossing Road Eastbound					Wolf's Crossing Road Westbound					Hoffman Boulevard Southbound					Int. Total
	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Right	Peds	App. Total	
7:00 AM	0	0	49	0	49	0	27	1	0	28	0	10	2	0	12	89
7:15 AM	0	1	51	0	52	0	28	1	0	29	0	10	6	0	16	97
7:30 AM	0	0	80	0	80	0	34	5	0	39	0	8	0	0	8	127
7:45 AM	0	1	95	0	96	0	27	2	0	29	0	9	2	0	11	136
Hourly Total	0	2	275	0	277	0	116	9	0	125	0	37	10	0	47	449
8:00 AM	0	1	57	0	58	0	27	5	0	32	0	8	2	0	10	100
8:15 AM	0	2	65	0	67	0	35	3	0	38	0	6	4	0	10	115
8:30 AM	0	1	71	0	72	0	37	2	0	39	0	6	5	0	11	122
8:45 AM	0	2	75	0	77	0	35	6	0	41	0	4	4	0	8	126
Hourly Total	0	6	268	0	274	0	134	16	0	150	0	24	15	0	39	463
*** BREAK ***	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
4:00 PM	0	5	73	0	78	0	92	12	0	104	0	8	0	0	8	190
4:15 PM	0	8	62	0	70	0	112	7	0	119	0	6	10	0	16	205
4:30 PM	0	5	60	0	65	0	99	6	0	105	0	9	7	0	16	186
4:45 PM	0	13	59	0	72	0	72	8	0	80	0	1	4	0	5	157
Hourly Total	0	31	254	0	285	0	375	33	0	408	0	24	21	0	45	738
5:00 PM	0	6	61	0	67	0	101	18	0	119	0	5	5	0	10	196
5:15 PM	0	8	64	0	72	0	89	13	0	102	0	5	3	0	8	182
5:30 PM	0	7	61	0	68	0	90	17	0	107	0	8	12	0	20	195
5:45 PM	0	5	47	0	52	0	87	15	0	102	0	3	3	0	6	160
Hourly Total	0	26	233	0	259	0	367	63	0	430	0	21	23	0	44	733
Grand Total	0	65	1030	0	1095	0	992	121	0	1113	0	106	69	0	175	2383
Approach %	0.0	5.9	94.1	-	-	0.0	89.1	10.9	-	-	0.0	60.6	39.4	-	-	-
Total %	0.0	2.7	43.2	-	46.0	0.0	41.6	5.1	-	46.7	0.0	4.4	2.9	-	7.3	-
Lights	0	65	995	-	1060	0	964	121	-	1085	0	105	69	-	174	2319
% Lights	-	100.0	96.6	-	96.8	-	97.2	100.0	-	97.5	-	99.1	100.0	-	99.4	97.3
Buses	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Buses	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Single-Unit Trucks	0	0	26	-	26	0	19	0	-	19	0	1	0	-	1	46
% Single-Unit Trucks	-	0.0	2.5	-	2.4	-	1.9	0.0	-	1.7	-	0.9	0.0	-	0.6	1.9
Articulated Trucks	0	0	9	-	9	0	9	0	-	9	0	0	0	-	0	18
% Articulated Trucks	-	0.0	0.9	-	0.8	-	0.9	0.0	-	0.8	-	0.0	0.0	-	0.0	0.8
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Start Time	Wolf's Crossing Road Eastbound					Wolf's Crossing Road Westbound					Hoffman Boulevard Southbound					Int. Total
	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Right	Peds	App. Total	
7:30 AM	0	0	80	0	80	0	34	5	0	39	0	8	0	0	8	127
7:45 AM	0	1	95	0	96	0	27	2	0	29	0	9	2	0	11	136
8:00 AM	0	1	57	0	58	0	27	5	0	32	0	8	2	0	10	100
8:15 AM	0	2	65	0	67	0	35	3	0	38	0	6	4	0	10	115
Total	0	4	297	0	301	0	123	15	0	138	0	31	8	0	39	478
Approach %	0.0	1.3	98.7	-	-	0.0	89.1	10.9	-	-	0.0	79.5	20.5	-	-	-
Total %	0.0	0.8	62.1	-	63.0	0.0	25.7	3.1	-	28.9	0.0	6.5	1.7	-	8.2	-
PHF	0.000	0.500	0.782	-	0.784	0.000	0.879	0.750	-	0.885	0.000	0.861	0.500	-	0.886	0.879
Lights	0	4	285	-	289	0	115	15	-	130	0	31	8	-	39	458
% Lights	-	100.0	96.0	-	96.0	-	93.5	100.0	-	94.2	-	100.0	100.0	-	100.0	95.8
Buses	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Buses	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Single-Unit Trucks	0	0	10	-	10	0	5	0	-	5	0	0	0	-	0	15
% Single-Unit Trucks	-	0.0	3.4	-	3.3	-	4.1	0.0	-	3.6	-	0.0	0.0	-	0.0	3.1
Articulated Trucks	0	0	2	-	2	0	3	0	-	3	0	0	0	-	0	5
% Articulated Trucks	-	0.0	0.7	-	0.7	-	2.4	0.0	-	2.2	-	0.0	0.0	-	0.0	1.0
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	-	0	0
% Bicycles on Road	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Pedestrians	-	-	-	0	-	-	-	-	0	-	-	-	-	0	-	-
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

Turning Movement Peak Hour Data (4:30 PM)

[illegible]

CMAP Traffic Projection Letter/ITE Data



Chicago Metropolitan
Agency for Planning

433 West Van Buren Street
Suite 450
Chicago, IL 60607

312-454-0400
cmap.illinois.gov

January 15, 2021

Javier Millan
Senior Consultant
Kenig, Lindgren, O'Hara and Aboona, Inc.
9575 West Higgins Road
Suite 400
Rosemont, IL 60018

Subject: US 30 - Eola Road - Wolfs Crossing Road -111th Street
IDOT

Dear Mr. Millan:

In response to a request made on your behalf and dated January 15, 2021, we have developed year 2050 average daily traffic (ADT) projections for the subject location.

ROAD SEGMENT	Current Volumes	Year 2050 ADT
US 30 north of Wolfs Crossing Rd	16,800	22,000
US 30 south of Wolfs Crossing Rd	20,000	23,200
Eola Rd north of Wolfs Crossing Rd	9,700	12,700
Eola Rd south of Wolfs Crossing Rd	3,900	4,500
Wolfs Crossing Rd west of US 30	8,650	13,500
111th St west of Lincoln Hwy	15,100	23,600

Traffic projections are developed using existing ADT data provided in the request letter and the results from the June 2020 CMAP Travel Demand Analysis. The regional travel model uses CMAP 2050 socioeconomic projections and assumes the implementation of the ON TO 2050 Comprehensive Regional Plan for the Northeastern Illinois area. The provision of this data in support of your request does not constitute a CMAP endorsement of the proposed development or any subsequent developments.

If you have any questions, please call me at (312) 386-8806.

Sincerely,

Jose Rodriguez, PTP, AICP
Senior Planner, Research & Analysis

cc: Quigley (IDOT)
2021_CY_TrafficForecast\Aurora\ke-01-21\ke-01-21.docx

Land Use: 251

Senior Adult Housing—Detached

Description

Senior adult housing consists of detached independent living developments, including retirement communities, age-restricted housing, and active adult communities. These developments may include amenities such as golf courses, swimming pools, 24-hour security, transportation, and common recreational facilities. However, they generally lack centralized dining and on-site health facilities. Detached senior adult housing communities may or may not be gated. Residents in these communities are typically active (requiring little to no medical supervision). The percentage of retired residents varies by development. Senior adult housing—attached (Land Use 252), congregate care facility (Land Use 253), assisted living (Land Use 254), and continuing care retirement community (Land Use 255) are related land uses.

Additional Data

Caution should be used when applying trip rates for this land use as it may contain a wide variety of studies ranging from communities with very active, working residents to communities with older, retired residents. As more data becomes available, consideration will be given to future stratification of this land use.

Many factors affected the trip rates for detached senior adult housing. Factors such as the average age of residents, development location and size, affluence of residents, employment status, and vehicular access should be taken into consideration when conducting an analysis. Some developments were located within close proximity to medical facilities, restaurants, shopping centers, banks, and recreational activities.

For the six sites for which the numbers of both total dwelling units and occupied dwelling units were available, an average of 98.5 percent of the total dwelling units were occupied.

Time-of-day distribution data for this land use are presented in Appendix A. For the six general urban/suburban sites with data, the overall highest vehicle volumes during the AM and PM on a weekday were counted between 11:45 a.m. and 12:45 p.m. and 5:00 and 6:00 p.m., respectively.

For the six sites for which data were provided for both occupied dwelling units and total dwelling units, an average of 98.5 percent of the units were occupied.

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in Alberta (CAN), California, Delaware, Florida, New Hampshire, New Jersey, and Pennsylvania.

Source Numbers

221, 289, 398, 421, 500, 550, 598, 601, 629, 734, 930

Senior Adult Housing - Detached (251)

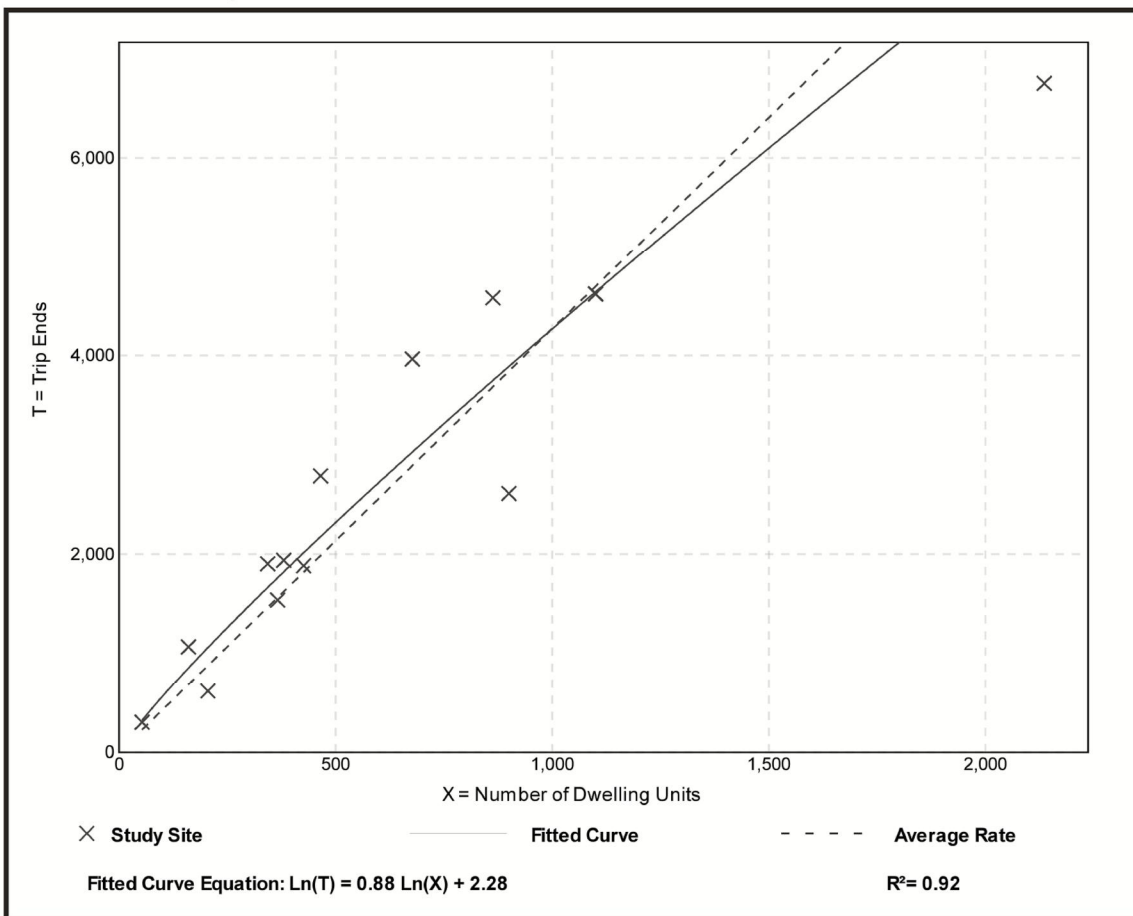
Vehicle Trip Ends vs: Dwelling Units
On a: Weekday

Setting/Location: General Urban/Suburban
Number of Studies: 14
Avg. Num. of Dwelling Units: 655
Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
4.27	2.90 - 6.66	1.11

Data Plot and Equation



Senior Adult Housing - Detached (251)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 29

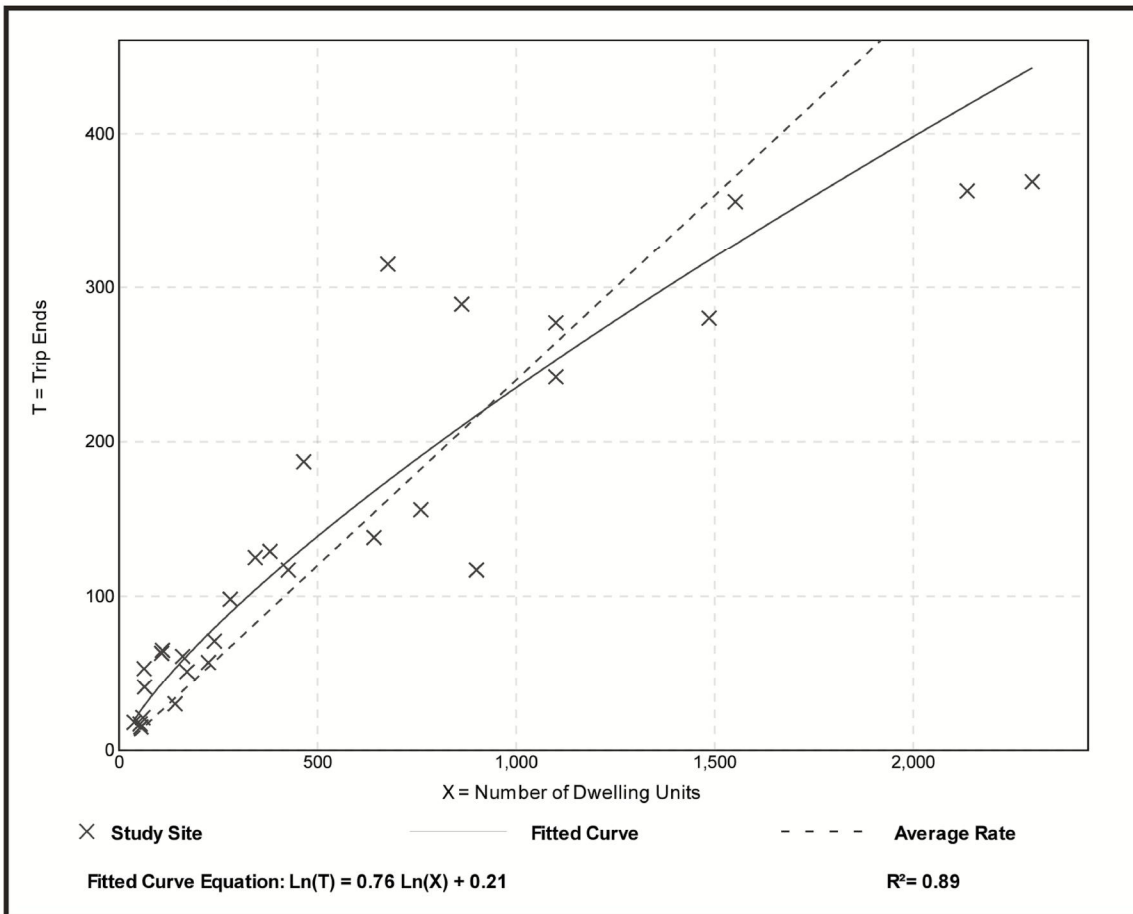
Avg. Num. of Dwelling Units: 583

Directional Distribution: 33% entering, 67% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.24	0.13 - 0.84	0.10

Data Plot and Equation



Senior Adult Housing - Detached (251)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 30

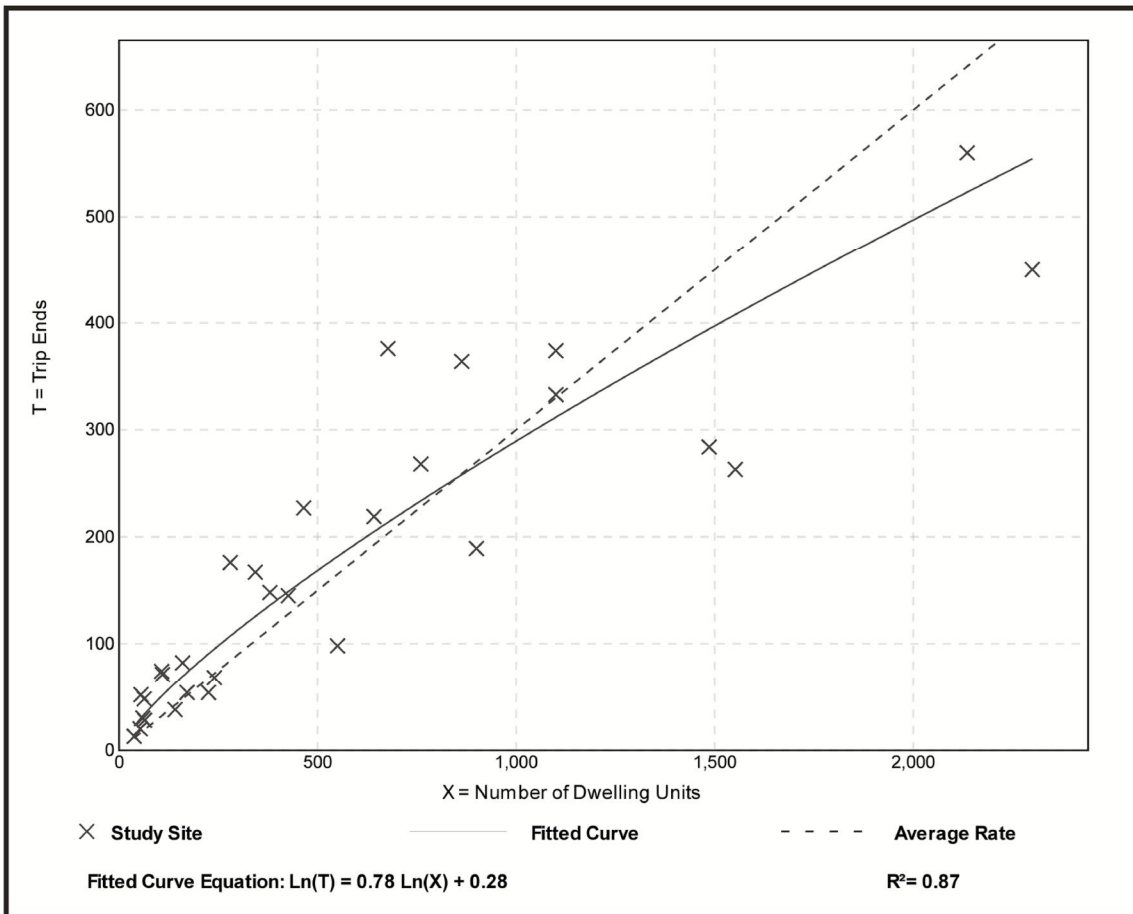
Avg. Num. of Dwelling Units: 582

Directional Distribution: 61% entering, 39% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.30	0.17 - 0.95	0.13

Data Plot and Equation



Land Use: 210

Single-Family Detached Housing

Description

Single-family detached housing includes all single-family detached homes on individual lots. A typical site surveyed is a suburban subdivision.

Additional Data

The number of vehicles and residents had a high correlation with average weekday vehicle trip ends. The use of these variables was limited, however, because the number of vehicles and residents was often difficult to obtain or predict. The number of dwelling units was generally used as the independent variable of choice because it was usually readily available, easy to project, and had a high correlation with average weekday vehicle trip ends.

This land use included data from a wide variety of units with different sizes, price ranges, locations, and ages. Consequently, there was a wide variation in trips generated within this category. Other factors, such as geographic location and type of adjacent and nearby development, may also have had an effect on the site trip generation.

Single-family detached units had the highest trip generation rate per dwelling unit of all residential uses because they were the largest units in size and had more residents and more vehicles per unit than other residential land uses; they were generally located farther away from shopping centers, employment areas, and other trip attractors than other residential land uses; and they generally had fewer alternative modes of transportation available because they were typically not as concentrated as other residential land uses.

Time-of-day distribution data for this land use are presented in Appendix A. For the six general urban/suburban sites with data, the overall highest vehicle volumes during the AM and PM on a weekday were counted between 7:15 and 8:15 a.m. and 4:00 and 5:00 p.m., respectively. For the two sites with Saturday data, the overall highest vehicle volume was counted between 3:00 and 4:00 p.m. For the one site with Sunday data, the overall highest vehicle volume was counted between 10:15 and 11:15 a.m.

The sites were surveyed in the 1980s, the 1990s, the 2000s, and the 2010s in California, Connecticut, Delaware, Illinois, Indiana, Maryland, Minnesota, Montana, New Jersey, North Carolina, Ohio, Oregon, Pennsylvania, South Carolina, South Dakota, Tennessee, Vermont, and Virginia.

Source Numbers

100, 105, 114, 126, 157, 167, 177, 197, 207, 211, 217, 267, 275, 293, 300, 319, 320, 356, 357, 367, 384, 387, 407, 435, 522, 550, 552, 579, 598, 601, 603, 614, 637, 711, 716, 720, 728, 735, 868, 903, 925, 936

Single-Family Detached Housing (210)

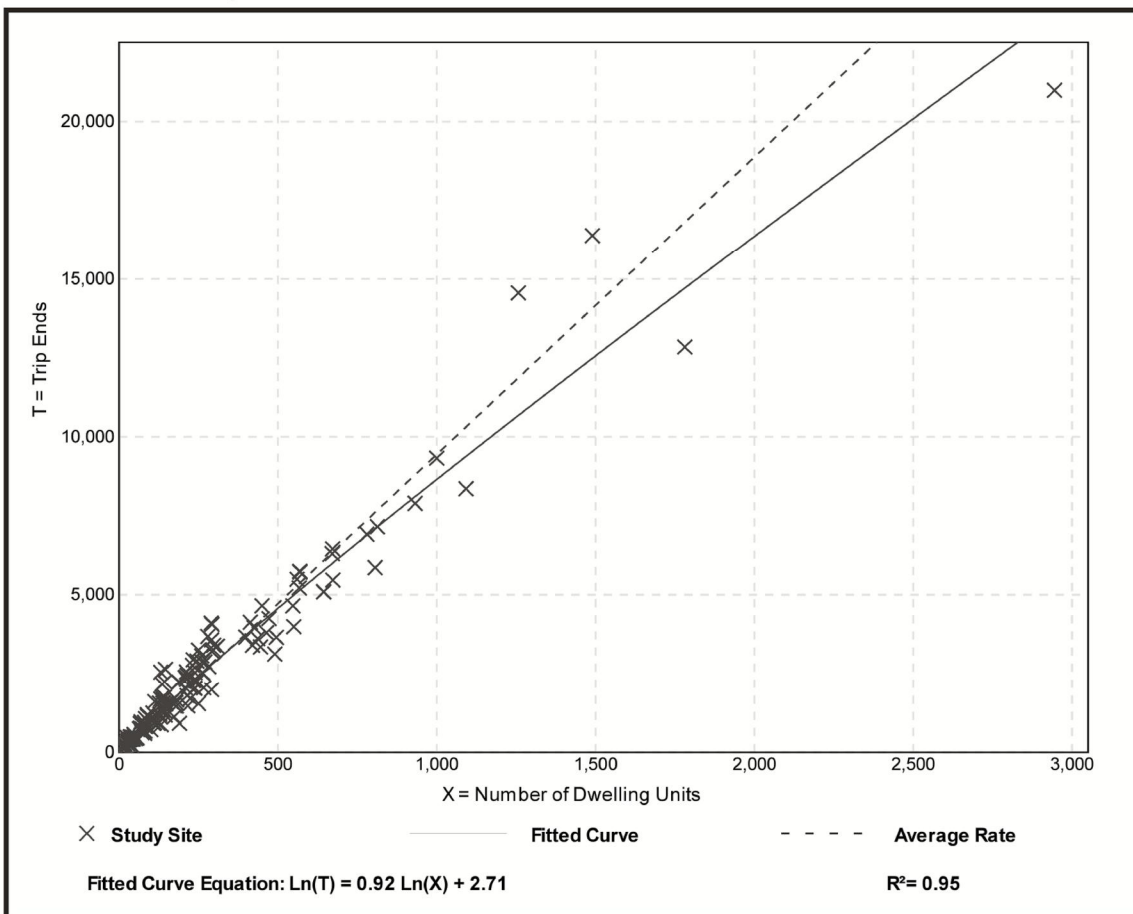
Vehicle Trip Ends vs: Dwelling Units
On a: Weekday

Setting/Location: General Urban/Suburban
Number of Studies: 159
Avg. Num. of Dwelling Units: 264
Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
9.44	4.81 - 19.39	2.10

Data Plot and Equation



Single-Family Detached Housing (210)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

Number of Studies: 173

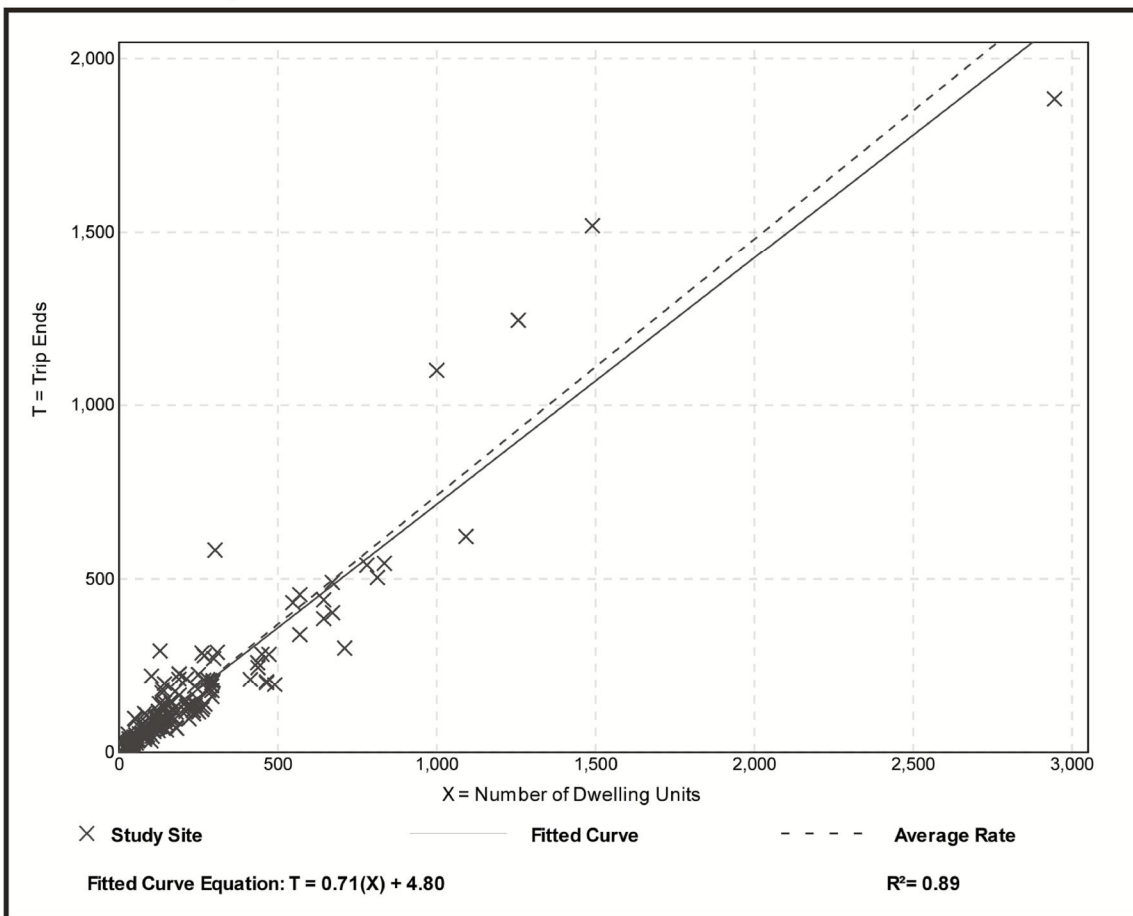
Avg. Num. of Dwelling Units: 219

Directional Distribution: 25% entering, 75% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.74	0.33 - 2.27	0.27

Data Plot and Equation



Single-Family Detached Housing (210)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

Number of Studies: 190

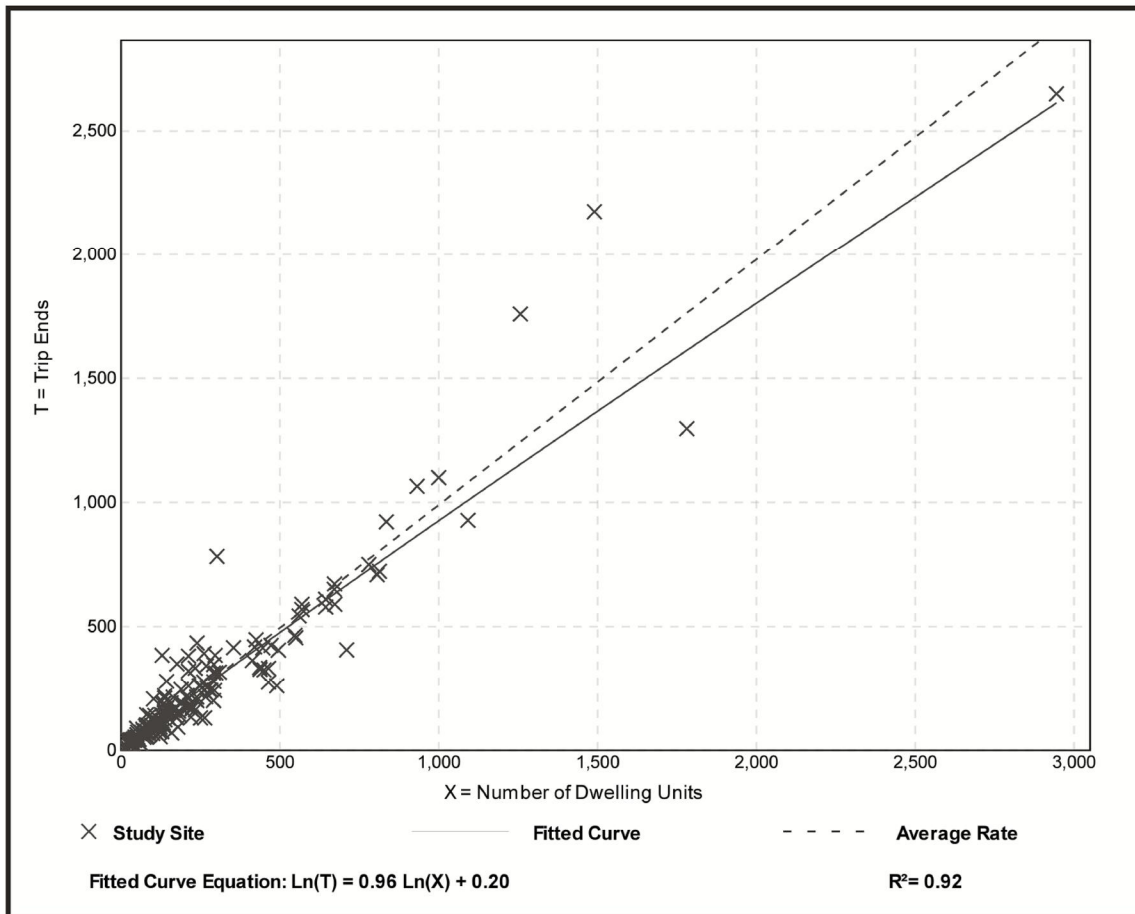
Avg. Num. of Dwelling Units: 242

Directional Distribution: 63% entering, 37% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.99	0.44 - 2.98	0.31

Data Plot and Equation



Shopping Center (820)

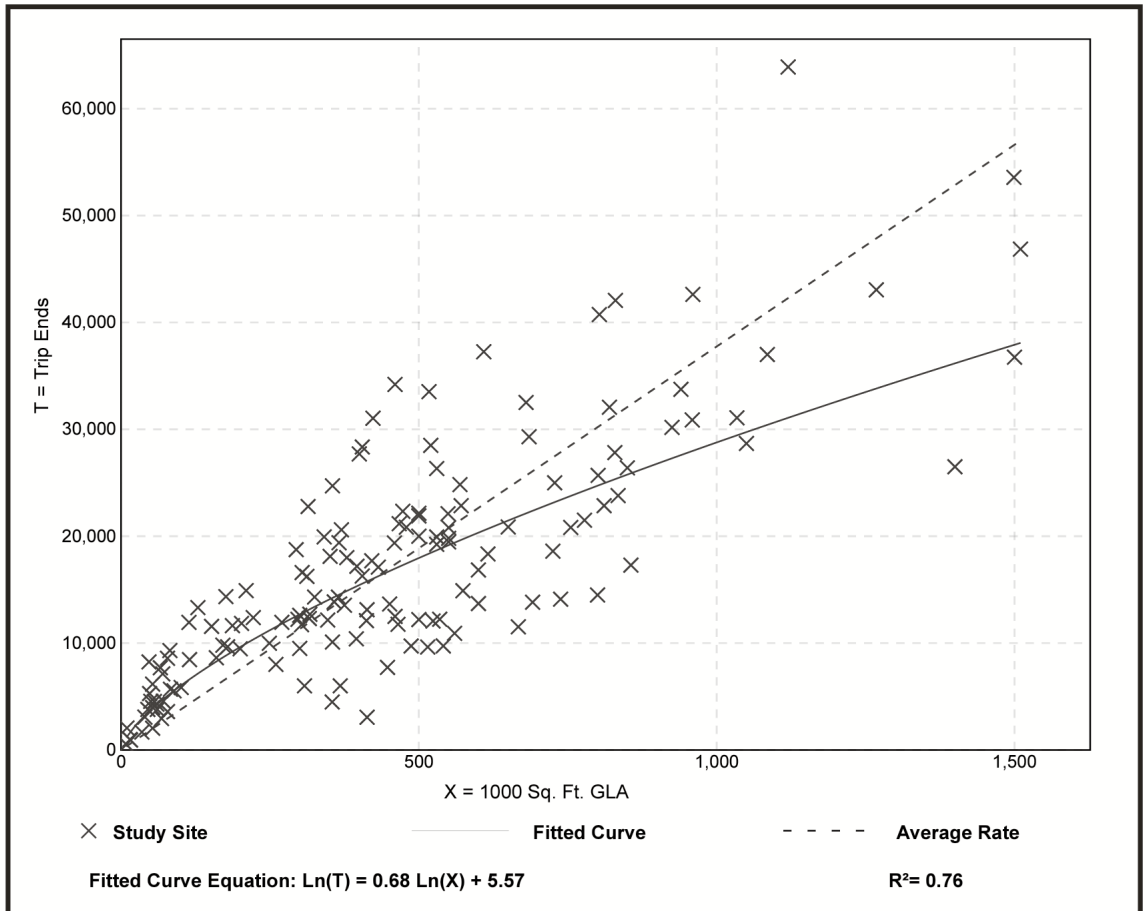
Vehicle Trip Ends vs: 1000 Sq. Ft. GLA
On a: Weekday

Setting/Location: General Urban/Suburban
Number of Studies: 147
1000 Sq. Ft. GLA: 453
Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation
37.75	7.42 - 207.98	16.41

Data Plot and Equation



Shopping Center (820)

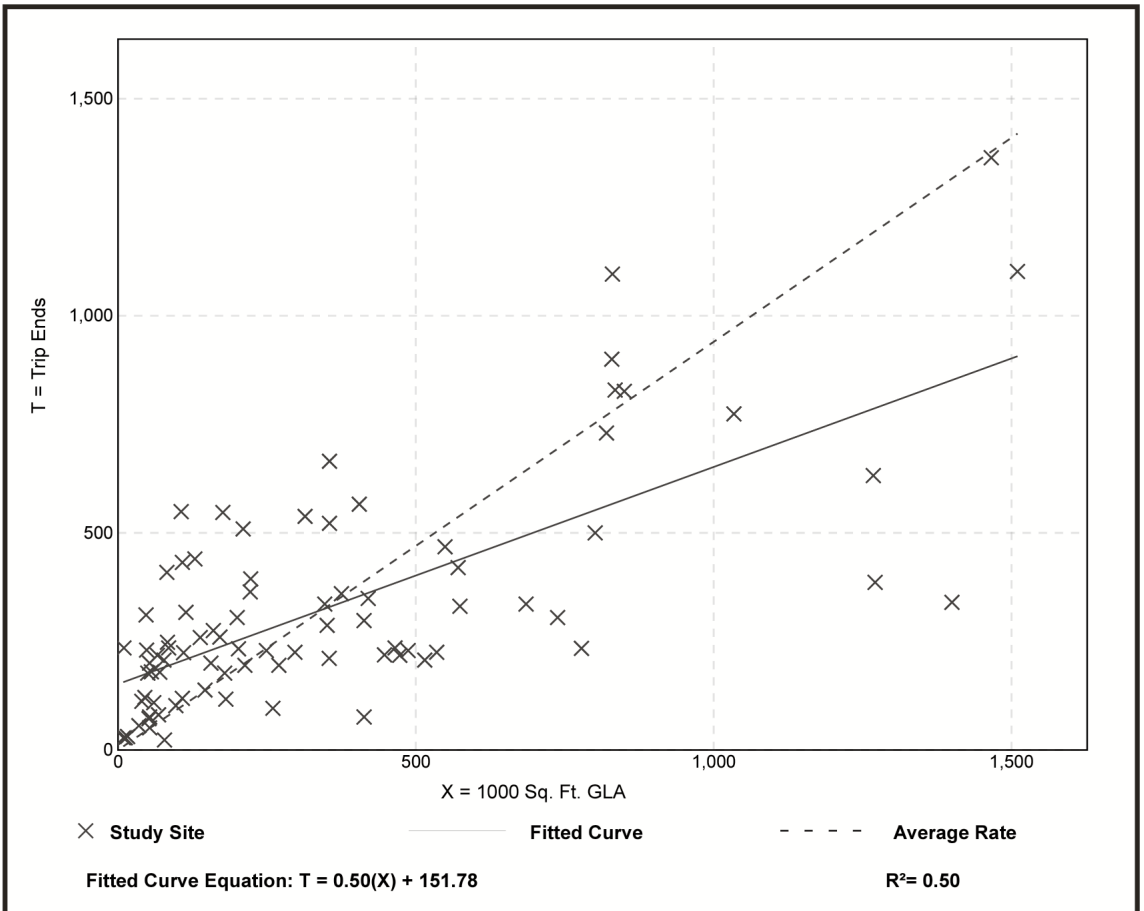
Vehicle Trip Ends vs: 1000 Sq. Ft. GLA
On a: Weekday,
Peak Hour of Adjacent Street Traffic,
One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban
Number of Studies: 84
1000 Sq. Ft. GLA: 351
Directional Distribution: 62% entering, 38% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation
0.94	0.18 - 23.74	0.87

Data Plot and Equation



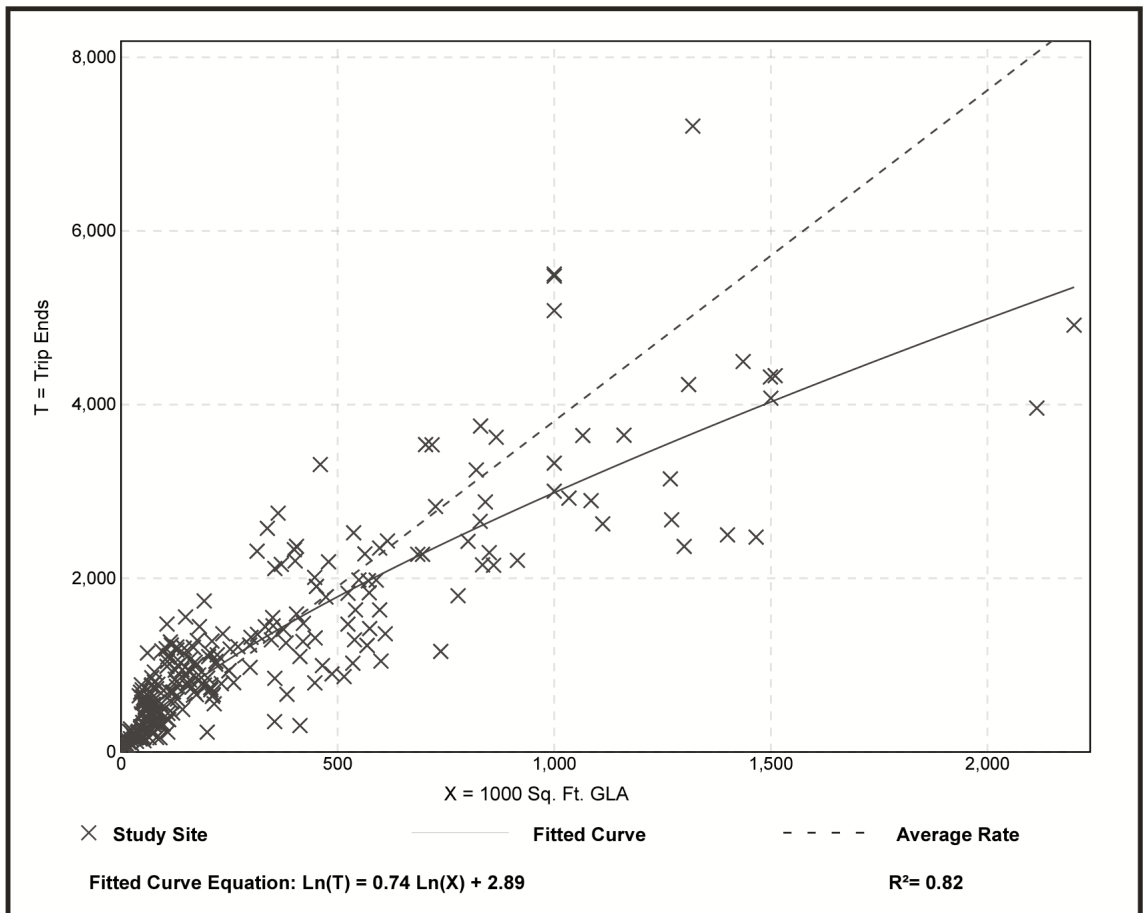
Shopping Center (820)

Vehicle Trip Ends vs: 1000 Sq. Ft. GLA
 On a: Weekday,
 Peak Hour of Adjacent Street Traffic,
 One Hour Between 4 and 6 p.m.
 Setting/Location: General Urban/Suburban
 Number of Studies: 261
 1000 Sq. Ft. GLA: 327
 Directional Distribution: 48% entering, 52% exiting

Vehicle Trip Generation per 1000 Sq. Ft. GLA

Average Rate	Range of Rates	Standard Deviation
3.81	0.74 - 18.69	2.04

Data Plot and Equation



Level of Service Criteria

LEVEL OF SERVICE CRITERIA

Signalized Intersections		
Level of Service	Interpretation	Average Control Delay (seconds per vehicle)
A	Favorable progression. Most vehicles arrive during the green indication and travel through the intersection without stopping.	≤10
B	Good progression, with more vehicles stopping than for Level of Service A.	>10 - 20
C	Individual cycle failures (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear. Number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.	>20 - 35
D	The volume-to-capacity ratio is high and either progression is ineffective or the cycle length is too long. Many vehicles stop and individual cycle failures are noticeable.	>35 - 55
E	Progression is unfavorable. The volume-to-capacity ratio is high and the cycle length is long. Individual cycle failures are frequent.	>55 - 80
F	The volume-to-capacity ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.	>80.0
Unsignalized Intersections		
Level of Service	Average Total Delay (SEC/VEH)	
A	0 - 10	
B	> 10 - 15	
C	> 15 - 25	
D	> 25 - 35	
E	> 35 - 50	
F	> 50	

Source: *Highway Capacity Manual*, 2010.

Capacity Analysis – Existing Conditions

Lanes, Volumes, Timings
1: US 30 & Wolf'S Crossing Road

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	40	363	83	59	206	119	42	725	7	97	649	6
Future Volume (vph)	40	363	83	59	206	119	42	725	7	97	649	6
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	0		0	280		0	130		0	282		0
Storage Lanes	0		0	1		0	1		0	1		0
Taper Length (ft)	25			255			112			180		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.977			0.945			0.999			0.999	
Flt Protected		0.996		0.950			0.950			0.950		
Satd. Flow (prot)	0	1808	0	1752	1705	0	1612	1694	0	1656	1725	0
Flt Permitted		0.844		0.286			0.193			0.108		
Satd. Flow (perm)	0	1532	0	528	1705	0	327	1694	0	188	1725	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		1099			2118			1789			736	
Travel Time (s)		25.0			48.1			40.7			16.7	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	3%	0%	3%	2%	11%	12%	12%	14%	9%	10%	17%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	511	0	62	342	0	44	770	0	102	689	0
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0		24.0	24.0		9.5	24.0		9.5	24.0	
Total Split (s)	38.0	38.0		38.0	38.0		9.6	52.4		9.6	52.4	
Total Split (%)	38.0%	38.0%		38.0%	38.0%		9.6%	52.4%		9.6%	52.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		3.5	4.0		3.5	4.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)		0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		6.0		6.0	6.0		3.5	6.0		3.5	6.0	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Recall Mode	None	None		None	None		None	Max		None	Max	
Act Effect Green (s)		32.1		32.1	32.1		53.8	46.5		54.5	48.4	
Actuated g/C Ratio		0.33		0.33	0.33		0.55	0.47		0.56	0.49	

Lanes, Volumes, Timings
1: US 30 & Wolf'S Crossing Road

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio		1.02		0.36	0.61		0.17	0.96		0.52	0.81	
Control Delay		80.5		33.8	34.1		10.5	50.2		19.8	31.4	
Queue Delay		0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay		80.5		33.8	34.1		10.5	50.2		19.8	31.4	
LOS		F		C	C		B	D		B	C	
Approach Delay		80.5			34.1			48.1			29.9	
Approach LOS		F			C			D			C	
Queue Length 50th (ft)		~355		30	184		11	467		27	380	
Queue Length 95th (ft)		#555		71	282		26	#730		53	#604	
Internal Link Dist (ft)		1019			2038			1709			656	
Turn Bay Length (ft)				280			130			282		
Base Capacity (vph)		500		172	557		259	803		195	850	
Starvation Cap Reductn		0		0	0		0	0		0	0	
Spillback Cap Reductn		0		0	0		0	0		0	0	
Storage Cap Reductn		0		0	0		0	0		0	0	
Reduced v/c Ratio		1.02		0.36	0.61		0.17	0.96		0.52	0.81	

Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 98.1

Natural Cycle: 100

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.02

Intersection Signal Delay: 46.7

Intersection LOS: D

Intersection Capacity Utilization 106.7%

ICU Level of Service G

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: US 30 & Wolf'S Crossing Road

 Ø1	 Ø2	 Ø4
9.6 s	52.4 s	38 s
 Ø5	 Ø6	 Ø8
9.6 s	52.4 s	38 s

Lanes, Volumes, Timings

2: Eola Road & Wolf'S Crossing Road

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	92	213	0	13	142	17	100	224	49	57	105	111
Future Volume (vph)	92	213	0	13	142	17	100	224	49	57	105	111
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	2000	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	185		0	260		0	165		0	220		0
Storage Lanes	1		0	1		0	1		0	1		1
Taper Length (ft)	140			220			180			170		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.984			0.973				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1787	1792	0	1570	1779	0	1805	3401	0	1805	1961	1568
Flt Permitted	0.512			0.619			0.646			0.575		
Satd. Flow (perm)	963	1792	0	1023	1779	0	1227	3401	0	1092	1961	1568
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		2118			1282			1814			1197	
Travel Time (s)		48.1			29.1			41.2			27.2	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	1%	6%	0%	15%	5%	6%	0%	4%	0%	0%	2%	3%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	97	224	0	14	167	0	105	288	0	60	111	117
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		6
Detector Phase	7	4		3	8		5	2		1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		9.5	24.0		9.5	24.0		9.5	24.0	24.0
Total Split (s)	11.0	29.0		10.0	28.0		11.0	31.0		10.0	30.0	30.0
Total Split (%)	13.8%	36.3%		12.5%	35.0%		13.8%	38.8%		12.5%	37.5%	37.5%
Yellow Time (s)	3.5	4.0		3.5	4.0		3.5	4.0		3.5	4.0	4.0
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	6.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	Yes
Recall Mode	None	None		None	None		None	C-Max		None	C-Max	C-Max
Act Effect Green (s)	24.3	20.3		20.9	13.5		46.7	39.1		44.6	36.4	36.4
Actuated g/C Ratio	0.30	0.25		0.26	0.17		0.58	0.49		0.56	0.46	0.46

Lanes, Volumes, Timings

2: Eola Road & Wolf'S Crossing Road

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.27	0.49		0.05	0.55		0.14	0.17		0.09	0.12	0.16
Control Delay	19.5	28.6		16.1	36.9		9.2	15.0		9.3	17.4	17.9
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	19.5	28.6		16.1	36.9		9.2	15.0		9.3	17.4	17.9
LOS	B	C		B	D		A	B		A	B	B
Approach Delay		25.8			35.3			13.5			15.9	
Approach LOS		C			D			B			B	
Queue Length 50th (ft)	34	89		5	78		21	45		12	33	36
Queue Length 95th (ft)	58	161		15	125		52	87		34	79	85
Internal Link Dist (ft)		2038			1202			1734			1117	
Turn Bay Length (ft)	185			260			165			220		
Base Capacity (vph)	370	531		316	489		775	1662		672	892	713
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.26	0.42		0.04	0.34		0.14	0.17		0.09	0.12	0.16

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.55

Intersection Signal Delay: 20.8

Intersection LOS: C

Intersection Capacity Utilization 44.0%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 2: Eola Road & Wolf'S Crossing Road

 Ø1	 Ø2 (R)	 Ø3	 Ø4
10 s	31 s	10 s	29 s
 Ø5	 Ø6 (R)	 Ø7	 Ø8
11 s	30 s	11 s	28 s

Lanes, Volumes, Timings

3: US 30 & Eola Road

01/21/2021

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	108	0	774	207	1	709
Future Volume (vph)	108	0	774	207	1	709
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)	0%		0%			0%
Storage Length (ft)	310	0		240	240	
Storage Lanes	1	1		1	1	
Taper Length (ft)	200				240	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt				0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1736	1900	1681	1583	1805	1712
Flt Permitted	0.950				0.255	
Satd. Flow (perm)	1736	1900	1681	1583	484	1712
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	30		30			30
Link Distance (ft)	660		2004			828
Travel Time (s)	15.0		45.5			18.8
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	4%	0%	13%	2%	0%	11%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%		0%			0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	114	0	815	218	1	746
Turn Type	Prot	Perm	NA	Perm	pm+pt	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2	6	
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	24.0	24.0	24.0	24.0	9.5	24.0
Total Split (s)	24.0	24.0	56.5	56.5	9.5	66.0
Total Split (%)	26.7%	26.7%	62.8%	62.8%	10.6%	73.3%
Yellow Time (s)	4.0	4.0	4.0	4.0	3.5	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	0.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	3.5	6.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Recall Mode	None	None	C-Max	C-Max	None	C-Max
Act Effect Green (s)	11.2		68.7	68.7	71.9	70.6
Actuated g/C Ratio	0.12		0.76	0.76	0.80	0.78

Lanes, Volumes, Timings

3: US 30 & Eola Road

01/21/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
v/c Ratio	0.53		0.63	0.18	0.00	0.56
Control Delay	45.1		11.3	5.4	3.0	7.5
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	45.1		11.3	5.4	3.0	7.5
LOS	D		B	A	A	A
Approach Delay	45.1		10.0			7.5
Approach LOS	D		B			A
Queue Length 50th (ft)	62		189	30	0	158
Queue Length 95th (ft)	109		#536	91	1	304
Internal Link Dist (ft)	580		1924			748
Turn Bay Length (ft)	310			240	240	
Base Capacity (vph)	347		1284	1209	474	1342
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.33		0.63	0.18	0.00	0.56

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.63

Intersection Signal Delay: 11.1

Intersection LOS: B

Intersection Capacity Utilization 56.7%

ICU Level of Service B

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: US 30 & Eola Road



Lanes, Volumes, Timings
4: US 30 & 11th Street

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	75	132	9	5	62	152	5	754	18	227	627	44
Future Volume (vph)	75	132	9	5	62	152	5	754	18	227	627	44
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	0		0	0		190	125		0	190		0
Storage Lanes	0		0	0		1	1		0	1		0
Taper Length (ft)	25			25			150			160		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.995				0.850		0.996			0.990	
Flt Protected		0.983			0.996		0.950			0.950		
Satd. Flow (prot)	0	1847	0	0	1815	1509	1805	1697	0	1736	1675	0
Flt Permitted		0.856			0.973		0.351			0.170		
Satd. Flow (perm)	0	1608	0	0	1773	1509	667	1697	0	311	1675	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		2372			910			1113			1927	
Travel Time (s)		53.9			20.7			25.3			43.8	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	1%	0%	60%	0%	7%	0%	11%	33%	4%	13%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	227	0	0	70	160	5	813	0	239	706	0
Turn Type	Perm	NA		Perm	NA	Perm	pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	5	2		1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0		24.0	24.0	24.0	9.5	24.0		9.5	24.0	
Total Split (s)	24.0	24.0		24.0	24.0	24.0	9.6	60.5		15.5	66.4	
Total Split (%)	24.0%	24.0%		24.0%	24.0%	24.0%	9.6%	60.5%		15.5%	66.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.5	4.0		3.5	4.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	0.0	2.0		0.0	2.0	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		6.0			6.0	6.0	3.5	6.0		3.5	6.0	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Recall Mode	None	None		None	None	None	None	Max		None	Max	
Act Effect Green (s)		16.5			16.5	16.5	62.7	54.6		70.5	66.2	
Actuated g/C Ratio		0.17			0.17	0.17	0.65	0.57		0.73	0.69	

Lanes, Volumes, Timings

4: US 30 & 11th Street

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio		0.83			0.23	0.62	0.01	0.85		0.64	0.61	
Control Delay		63.8			37.0	48.8	4.4	28.8		14.2	12.2	
Queue Delay		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		63.8			37.0	48.8	4.4	28.8		14.2	12.2	
LOS		E			D	D	A	C		B	B	
Approach Delay		63.8			45.2			28.6			12.7	
Approach LOS		E			D			C			B	
Queue Length 50th (ft)		136			37	92	1	405		41	204	
Queue Length 95th (ft)		#258			79	164	4	#697		88	420	
Internal Link Dist (ft)		2292			830			1033			1847	
Turn Bay Length (ft)						190	125			190		
Base Capacity (vph)		300			331	281	509	959		404	1148	
Starvation Cap Reductn		0			0	0	0	0		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.76			0.21	0.57	0.01	0.85		0.59	0.61	

Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 96.5

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.85

Intersection Signal Delay: 27.2

Intersection LOS: C

Intersection Capacity Utilization 85.0%

ICU Level of Service E

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 4: US 30 & 11th Street

 Ø1	 Ø2	 Ø4
15.5 s	60.5 s	24 s
 Ø5	 Ø6	 Ø8
9.6 s	66.4 s	24 s

HCM 6th TWSC
5: Wolf'S Crossing Road & Hoffman Blvd.

01/21/2021

Intersection

Int Delay, s/veh 0.9

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	4	315	164	15	31	8
Future Vol, veh/h	4	315	164	15	31	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	8	5	0	0	0
Mvmt Flow	4	332	173	16	33	8

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	189	0	0 521 181
Stage 1	-	-	- - 181 -
Stage 2	-	-	- - 340 -
Critical Hdwy	4.1	-	- - 6.4 6.2
Critical Hdwy Stg 1	-	-	- - 5.4 -
Critical Hdwy Stg 2	-	-	- - 5.4 -
Follow-up Hdwy	2.2	-	- - 3.5 3.3
Pot Cap-1 Maneuver	1397	-	- - 519 867
Stage 1	-	-	- - 855 -
Stage 2	-	-	- - 725 -
Platoon blocked, %		-	- -
Mov Cap-1 Maneuver	1397	-	- - 517 867
Mov Cap-2 Maneuver	-	-	- - 517 -
Stage 1	-	-	- - 852 -
Stage 2	-	-	- - 725 -

Approach	EB	WB	SB
HCM Control Delay, s	0.1	0	11.9
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1397	-	-	-	564
HCM Lane V/C Ratio	0.003	-	-	-	0.073
HCM Control Delay (s)	7.6	-	-	-	11.9
HCM Lane LOS	A	-	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	0.2

Lanes, Volumes, Timings
1: US 30 & Wolf'S Crossing Road

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	15	213	66	81	340	187	124	653	12	113	699	17
Future Volume (vph)	15	213	66	81	340	187	124	653	12	113	699	17
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	0		0	280		0	130		0	282		0
Storage Lanes	0		0	1		0	1		0	1		0
Taper Length (ft)	25			255			112			180		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.970			0.947			0.997			0.996	
Flt Protected		0.997		0.950			0.950			0.950		
Satd. Flow (prot)	0	1837	0	1805	1763	0	1805	1789	0	1752	1804	0
Flt Permitted		0.633		0.448			0.108			0.153		
Satd. Flow (perm)	0	1167	0	851	1763	0	205	1789	0	282	1804	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		1099			2118			1789			736	
Travel Time (s)		25.0			48.1			40.7			16.7	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	1%	4%	0%	6%	0%	3%	5%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	309	0	85	555	0	131	700	0	119	754	0
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0		24.0	24.0		9.5	24.0		9.5	24.0	
Total Split (s)	38.0	38.0		38.0	38.0		10.0	52.0		10.0	52.0	
Total Split (%)	38.0%	38.0%		38.0%	38.0%		10.0%	52.0%		10.0%	52.0%	
Yellow Time (s)	4.0	4.0		4.0	4.0		3.5	4.0		3.5	4.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)		0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		6.0		6.0	6.0		3.5	6.0		3.5	6.0	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Recall Mode	None	None		None	None		None	Max		None	Max	
Act Effect Green (s)		32.0		32.0	32.0		55.0	46.0		55.0	46.0	
Actuated g/C Ratio		0.32		0.32	0.32		0.55	0.46		0.55	0.46	

Lanes, Volumes, Timings
1: US 30 & Wolf'S Crossing Road

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio		0.83		0.31	0.98		0.61	0.85		0.48	0.91	
Control Delay		51.8		29.7	69.3		23.5	35.8		16.0	42.1	
Queue Delay		0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay		51.8		29.7	69.3		23.5	35.8		16.0	42.1	
LOS		D		C	E		C	D		B	D	
Approach Delay		51.8			64.1			33.9			38.5	
Approach LOS		D			E			C			D	
Queue Length 50th (ft)		181		41	349		35	384		32	432	
Queue Length 95th (ft)		#330		84	#570		#76	#605		57	#678	
Internal Link Dist (ft)		1019			2038			1709			656	
Turn Bay Length (ft)				280			130			282		
Base Capacity (vph)		373		272	564		216	823		250	829	
Starvation Cap Reductn		0		0	0		0	0		0	0	
Spillback Cap Reductn		0		0	0		0	0		0	0	
Storage Cap Reductn		0		0	0		0	0		0	0	
Reduced v/c Ratio		0.83		0.31	0.98		0.61	0.85		0.48	0.91	

Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 100

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.98

Intersection Signal Delay: 44.8

Intersection LOS: D

Intersection Capacity Utilization 96.2%

ICU Level of Service F

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: US 30 & Wolf'S Crossing Road

 Ø1	 Ø2	 Ø4
10 s	52 s	38 s
 Ø5	 Ø6	 Ø8
10 s	52 s	38 s

Lanes, Volumes, Timings

2: Eola Road & Wolf'S Crossing Road

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	112	201	1	36	372	82	42	206	34	50	207	143
Future Volume (vph)	112	201	1	36	372	82	42	206	34	50	207	143
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	2000	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	185		0	260		0	165		0	220		0
Storage Lanes	1		0	1		0	1		0	1		1
Taper Length (ft)	140			220			180			170		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.999			0.973			0.979				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1861	0	1626	1804	0	1805	3441	0	1770	1980	1599
Flt Permitted	0.230			0.625			0.587			0.593		
Satd. Flow (perm)	437	1861	0	1070	1804	0	1115	3441	0	1105	1980	1599
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		2118			1282			1814			1197	
Travel Time (s)		48.1			29.1			41.2			27.2	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	2%	0%	11%	3%	0%	0%	1%	13%	2%	1%	1%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	118	213	0	38	478	0	44	253	0	53	218	151
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		6
Detector Phase	7	4		3	8		5	2		1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		9.5	24.0		9.5	24.0		9.5	24.0	24.0
Total Split (s)	10.0	36.0		10.0	36.0		10.0	24.0		10.0	24.0	24.0
Total Split (%)	12.5%	45.0%		12.5%	45.0%		12.5%	30.0%		12.5%	30.0%	30.0%
Yellow Time (s)	3.5	4.0		3.5	4.0		3.5	4.0		3.5	4.0	4.0
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	6.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	Yes
Recall Mode	None	None		None	None		None	C-Max		None	C-Max	C-Max
Act Effect Green (s)	34.1	27.7		33.3	25.7		34.7	28.2		34.8	28.3	28.3
Actuated g/C Ratio	0.43	0.35		0.42	0.32		0.43	0.35		0.44	0.35	0.35

Lanes, Volumes, Timings

2: Eola Road & Wolf'S Crossing Road

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.40	0.33		0.08	0.83		0.08	0.21		0.10	0.31	0.27
Control Delay	15.0	20.3		10.1	37.4		15.6	22.6		15.7	24.8	25.0
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	15.0	20.3		10.1	37.4		15.6	22.6		15.7	24.8	25.0
LOS	B	C		B	D		B	C		B	C	C
Approach Delay		18.4			35.4			21.6			23.7	
Approach LOS		B			D			C			C	
Queue Length 50th (ft)	30	79		9	213		13	53		15	91	62
Queue Length 95th (ft)	53	126		22	308		34	88		39	161	120
Internal Link Dist (ft)		2038			1202			1734			1117	
Turn Bay Length (ft)	185			260			165			220		
Base Capacity (vph)	297	712		491	676		539	1213		535	700	565
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.40	0.30		0.08	0.71		0.08	0.21		0.10	0.31	0.27

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.83

Intersection Signal Delay: 26.0

Intersection LOS: C

Intersection Capacity Utilization 61.9%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 2: Eola Road & Wolf'S Crossing Road

				
Ø1	Ø2 (R)		Ø3	Ø4
10 s	24 s		10 s	36 s
				
Ø5	Ø6 (R)		Ø7	Ø8
10 s	24 s		10 s	36 s

Lanes, Volumes, Timings

3: US 30 & Eola Road

01/21/2021

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	232	1	788	218	2	844
Future Volume (vph)	232	1	788	218	2	844
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12
Grade (%)	0%		0%			0%
Storage Length (ft)	310	0		240	240	
Storage Lanes	1	1		1	1	
Taper Length (ft)	200				240	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt		0.850		0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1787	1615	1810	1599	1805	1810
Flt Permitted	0.950				0.207	
Satd. Flow (perm)	1787	1615	1810	1599	393	1810
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	30		30			30
Link Distance (ft)	660		2004			828
Travel Time (s)	15.0		45.5			18.8
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	1%	0%	5%	1%	0%	5%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%		0%			0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	244	1	829	229	2	888
Turn Type	Prot	Perm	NA	Perm	pm+pt	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2	6	
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	24.0	24.0	24.0	24.0	9.5	24.0
Total Split (s)	24.0	24.0	56.5	56.5	9.5	66.0
Total Split (%)	26.7%	26.7%	62.8%	62.8%	10.6%	73.3%
Yellow Time (s)	4.0	4.0	4.0	4.0	3.5	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	0.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	3.5	6.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Recall Mode	None	None	C-Max	C-Max	None	C-Max
Act Effect Green (s)	16.0	16.0	60.2	60.2	64.5	62.0
Actuated g/C Ratio	0.18	0.18	0.67	0.67	0.72	0.69

Lanes, Volumes, Timings

3: US 30 & Eola Road

01/21/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
v/c Ratio	0.77	0.00	0.69	0.21	0.01	0.71
Control Delay	51.8	29.0	14.5	7.4	4.5	13.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.1
Total Delay	51.8	29.0	14.5	7.4	4.5	13.3
LOS	D	C	B	A	A	B
Approach Delay	51.7		12.9			13.2
Approach LOS	D		B			B
Queue Length 50th (ft)	131	0	250	44	0	285
Queue Length 95th (ft)	#217	5	533	102	2	445
Internal Link Dist (ft)	580		1924			748
Turn Bay Length (ft)	310			240	240	
Base Capacity (vph)	357	323	1210	1069	376	1247
Starvation Cap Reductn	0	0	0	0	0	25
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.68	0.00	0.69	0.21	0.01	0.73

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.77

Intersection Signal Delay: 17.4

Intersection LOS: B

Intersection Capacity Utilization 67.3%

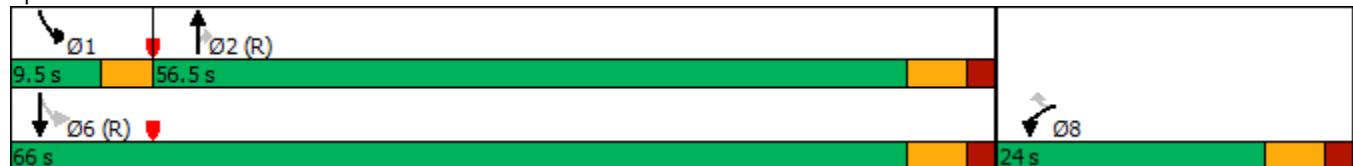
ICU Level of Service C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: US 30 & Eola Road



Lanes, Volumes, Timings
4: US 30 & 11th Street

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	71	62	6	16	187	276	17	659	9	162	803	111
Future Volume (vph)	71	62	6	16	187	276	17	659	9	162	803	111
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	0		0	0		190	125		0	190		0
Storage Lanes	0		0	0		1	1		0	1		0
Taper Length (ft)	25			25			150			160		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.994				0.850		0.998			0.982	
Flt Protected		0.975			0.996		0.950			0.950		
Satd. Flow (prot)	0	1833	0	0	1806	1509	1805	1704	0	1736	1671	0
Flt Permitted		0.513			0.970		0.156			0.255		
Satd. Flow (perm)	0	965	0	0	1759	1509	296	1704	0	466	1671	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		2372			910			1113			1927	
Travel Time (s)		53.9			20.7			25.3			43.8	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	1%	0%	60%	0%	7%	0%	11%	33%	4%	13%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	146	0	0	214	291	18	703	0	171	962	0
Turn Type	Perm	NA		Perm	NA	Perm	pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	5	2		1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0		24.0	24.0	24.0	9.5	24.0		9.5	24.0	
Total Split (s)	24.0	24.0		24.0	24.0	24.0	9.6	65.6		10.4	66.4	
Total Split (%)	24.0%	24.0%		24.0%	24.0%	24.0%	9.6%	65.6%		10.4%	66.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.5	4.0		3.5	4.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	0.0	2.0		0.0	2.0	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		6.0			6.0	6.0	3.5	6.0		3.5	6.0	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Recall Mode	None	None		None	None	None	None	Max		None	Max	
Act Effect Green (s)		18.0			18.0	18.0	67.8	59.6		71.4	66.1	
Actuated g/C Ratio		0.18			0.18	0.18	0.68	0.60		0.71	0.66	

Lanes, Volumes, Timings

4: US 30 & 11th Street

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio		0.84			0.68	1.07	0.06	0.69		0.41	0.87	
Control Delay		78.5			50.2	116.0	4.4	18.4		7.2	25.7	
Queue Delay		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		78.5			50.2	116.0	4.4	18.4		7.2	25.7	
LOS		E			D	F	A	B		A	C	
Approach Delay		78.5			88.1			18.1			22.9	
Approach LOS		E			F			B			C	
Queue Length 50th (ft)		91			129	~207	3	286		29	379	
Queue Length 95th (ft)		#203			#212	#370	8	424		48	#849	
Internal Link Dist (ft)		2292			830			1033			1847	
Turn Bay Length (ft)						190	125			190		
Base Capacity (vph)		173			316	271	293	1016		420	1105	
Starvation Cap Reductn		0			0	0	0	0		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.84			0.68	1.07	0.06	0.69		0.41	0.87	

Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 99.9

Natural Cycle: 90

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.07

Intersection Signal Delay: 37.9

Intersection LOS: D

Intersection Capacity Utilization 89.8%

ICU Level of Service E

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 4: US 30 & 11th Street

 Ø1	 Ø2	 Ø4
10.4 s	65.6 s	24 s
 Ø5	 Ø6	 Ø8
9.6 s	66.4 s	24 s

HCM 6th TWSC
5: Wolf'S Crossing Road & Hoffman Blvd.

01/21/2021

Intersection

Int Delay, s/veh 1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	32	253	471	45	20	19
Future Vol, veh/h	32	253	471	45	20	19
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	8	5	0	0	0
Mvmt Flow	34	266	496	47	21	20

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	543	0	854
Stage 1	-	-	520
Stage 2	-	-	334
Critical Hdwy	4.1	-	6.4
Critical Hdwy Stg 1	-	-	5.4
Critical Hdwy Stg 2	-	-	5.4
Follow-up Hdwy	2.2	-	3.5
Pot Cap-1 Maneuver	1036	-	332
Stage 1	-	-	601
Stage 2	-	-	730
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	1036	-	321
Mov Cap-2 Maneuver	-	-	321
Stage 1	-	-	581
Stage 2	-	-	730

Approach	EB	WB	SB
HCM Control Delay, s	1	0	14.9
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1036	-	-	-	405
HCM Lane V/C Ratio	0.033	-	-	-	0.101
HCM Control Delay (s)	8.6	-	-	-	14.9
HCM Lane LOS	A	-	-	-	B
HCM 95th %tile Q(veh)	0.1	-	-	-	0.3

Capacity Analysis – Year 2036 Base Projected Conditions

Lanes, Volumes, Timings
1: US 30 & Wolf'S Crossing Road

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	46	417	95	68	237	137	48	865	8	112	746	7
Future Volume (vph)	46	417	95	68	237	137	48	865	8	112	746	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	0		0	280		0	130		0	282		0
Storage Lanes	0		0	1		0	1		0	1		0
Taper Length (ft)	25			255			112			180		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.977			0.945			0.999			0.999	
Flt Protected		0.996		0.950			0.950			0.950		
Satd. Flow (prot)	0	1808	0	1752	1705	0	1612	1694	0	1656	1725	0
Flt Permitted		0.708		0.233			0.102			0.083		
Satd. Flow (perm)	0	1285	0	430	1705	0	173	1694	0	145	1725	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		1099			2118			1789			736	
Travel Time (s)		25.0			48.1			40.7			16.7	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	3%	0%	3%	2%	11%	12%	12%	14%	9%	10%	17%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	587	0	72	393	0	51	919	0	118	792	0
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0		24.0	24.0		9.5	24.0		9.5	24.0	
Total Split (s)	38.0	38.0		38.0	38.0		9.6	52.4		9.6	52.4	
Total Split (%)	38.0%	38.0%		38.0%	38.0%		9.6%	52.4%		9.6%	52.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		3.5	4.0		3.5	4.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)		0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		6.0		6.0	6.0		3.5	6.0		3.5	6.0	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Recall Mode	None	None		None	None		None	Max		None	Max	
Act Effect Green (s)		32.0		32.0	32.0		54.9	46.4		55.7	48.3	
Actuated g/C Ratio		0.32		0.32	0.32		0.55	0.46		0.56	0.48	

Lanes, Volumes, Timings

1: US 30 & Wolf'S Crossing Road

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio		1.43		0.53	0.72		0.28	1.17		0.69	0.95	
Control Delay		235.6		44.1	38.9		13.0	116.9		35.9	48.2	
Queue Delay		0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay		235.6		44.1	38.9		13.0	116.9		35.9	48.2	
LOS		F		D	D		B	F		D	D	
Approach Delay		235.6			39.7			111.4			46.6	
Approach LOS		F			D			F			D	
Queue Length 50th (ft)		~509		37	220		13	~702		31	484	
Queue Length 95th (ft)		#717		#92	331		29	#938		#108	#751	
Internal Link Dist (ft)		1019			2038			1709			656	
Turn Bay Length (ft)				280			130			282		
Base Capacity (vph)		411		137	545		182	786		172	833	
Starvation Cap Reductn		0		0	0		0	0		0	0	
Spillback Cap Reductn		0		0	0		0	0		0	0	
Storage Cap Reductn		0		0	0		0	0		0	0	
Reduced v/c Ratio		1.43		0.53	0.72		0.28	1.17		0.69	0.95	

Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 100

Natural Cycle: 150

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.43

Intersection Signal Delay: 104.8

Intersection LOS: F

Intersection Capacity Utilization 121.6%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: US 30 & Wolf'S Crossing Road

 Ø1	 Ø2	 Ø4
9.6 s	52.4 s	38 s
 Ø5	 Ø6	 Ø8
9.6 s	52.4 s	38 s

Lanes, Volumes, Timings

2: Eola Road & Wolf'S Crossing Road

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	106	245	0	15	163	20	115	258	56	66	121	128
Future Volume (vph)	106	245	0	15	163	20	115	258	56	66	121	128
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	2000	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	185		0	260		0	165		0	220		0
Storage Lanes	1		0	1		0	1		0	1		1
Taper Length (ft)	140			220			180			170		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor												
Frt					0.984			0.973				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1787	1792	0	1570	1779	0	1805	3401	0	1805	1961	1568
Flt Permitted	0.478			0.600			0.657			0.552		
Satd. Flow (perm)	899	1792	0	991	1779	0	1248	3401	0	1049	1961	1568
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		2118			1282			1814			1197	
Travel Time (s)		48.1			29.1			41.2			27.2	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	1%	6%	0%	15%	5%	6%	0%	4%	0%	0%	2%	3%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	112	258	0	16	193	0	121	331	0	69	127	135
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		6
Detector Phase	7	4		3	8		5	2		1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		9.5	24.0		9.5	24.0		9.5	24.0	24.0
Total Split (s)	11.0	29.0		10.0	28.0		11.0	31.0		10.0	30.0	30.0
Total Split (%)	13.8%	36.3%		12.5%	35.0%		13.8%	38.8%		12.5%	37.5%	37.5%
Yellow Time (s)	3.5	4.0		3.5	4.0		3.5	4.0		3.5	4.0	4.0
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	6.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	Yes
Recall Mode	None	None		None	None		None	C-Max		None	C-Max	C-Max
Act Effect Green (s)	25.6	21.6		22.2	14.8		45.0	36.1		43.2	35.2	35.2
Actuated g/C Ratio	0.32	0.27		0.28	0.18		0.56	0.45		0.54	0.44	0.44

Lanes, Volumes, Timings

2: Eola Road & Wolf'S Crossing Road

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.30	0.53		0.05	0.59		0.16	0.22		0.11	0.15	0.20
Control Delay	19.3	28.5		15.3	36.5		10.0	16.7		10.1	18.1	18.8
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	19.3	28.5		15.3	36.5		10.0	16.7		10.1	18.1	18.8
LOS	B	C		B	D		B	B		B	B	B
Approach Delay		25.7			34.9			14.9			16.7	
Approach LOS		C			C			B			B	
Queue Length 50th (ft)	39	103		5	90		26	55		14	40	43
Queue Length 95th (ft)	63	180		15	137		62	100		39	89	97
Internal Link Dist (ft)		2038			1202			1734			1117	
Turn Bay Length (ft)	185			260			165			220		
Base Capacity (vph)	370	542		325	489		758	1533		632	863	690
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.30	0.48		0.05	0.39		0.16	0.22		0.11	0.15	0.20

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.59

Intersection Signal Delay: 21.3

Intersection LOS: C

Intersection Capacity Utilization 46.8%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 2: Eola Road & Wolf'S Crossing Road

 Ø1	 Ø2 (R)	 Ø3	 Ø4
10 s	31 s	10 s	29 s
 Ø5	 Ø6 (R)	 Ø7	 Ø8
11 s	30 s	11 s	28 s

Lanes, Volumes, Timings

3: US 30 & Eola Road

01/21/2021

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	124	0	921	238	1	908
Future Volume (vph)	124	0	921	238	1	908
Ideal Flow (vphpl)	1900	1900	2000	1900	1900	2000
Lane Width (ft)	12	12	12	12	12	12
Grade (%)	0%		0%			0%
Storage Length (ft)	310	0		240	240	
Storage Lanes	1	1		1	1	
Taper Length (ft)	200				240	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt				0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1736	1900	1770	1583	1805	1802
Flt Permitted	0.950				0.162	
Satd. Flow (perm)	1736	1900	1770	1583	308	1802
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	30		30			30
Link Distance (ft)	660		2004			828
Travel Time (s)	15.0		45.5			18.8
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	4%	0%	13%	2%	0%	11%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%		0%			0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	131	0	969	251	1	956
Turn Type	Prot	Perm	NA	Perm	pm+pt	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2	6	
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	24.0	24.0	24.0	24.0	9.5	24.0
Total Split (s)	24.0	24.0	56.0	56.0	10.0	66.0
Total Split (%)	26.7%	26.7%	62.2%	62.2%	11.1%	73.3%
Yellow Time (s)	4.0	4.0	4.0	4.0	3.5	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	0.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	3.5	6.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Recall Mode	None	None	C-Max	C-Max	None	C-Max
Act Effect Green (s)	12.1		64.1	64.1	68.4	65.9
Actuated g/C Ratio	0.13		0.71	0.71	0.76	0.73

Lanes, Volumes, Timings
3: US 30 & Eola Road

01/21/2021

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
v/c Ratio	0.56		0.77	0.22	0.00	0.72
Control Delay	45.3		16.0	6.2	4.0	11.8
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	45.3		16.0	6.2	4.0	11.8
LOS	D		B	A	A	B
Approach Delay	45.3		14.0			11.8
Approach LOS	D		B			B
Queue Length 50th (ft)	71		270	38	0	257
Queue Length 95th (ft)	121		#778	108	1	502
Internal Link Dist (ft)	580		1924			748
Turn Bay Length (ft)	310			240	240	
Base Capacity (vph)	347		1261	1127	342	1320
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.38		0.77	0.22	0.00	0.72

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.77

Intersection Signal Delay: 14.8

Intersection LOS: B

Intersection Capacity Utilization 62.9%

ICU Level of Service B

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: US 30 & Eola Road



Lanes, Volumes, Timings
4: US 30 & 11th Street

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	86	152	10	6	71	175	6	898	21	261	408	51
Future Volume (vph)	86	152	10	6	71	175	6	898	21	261	408	51
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	0		0	0		190	125		0	190		0
Storage Lanes	0		0	0		1	1		0	1		0
Taper Length (ft)	25			25			150			160		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.994				0.850		0.997			0.983	
Flt Protected		0.983			0.996		0.950			0.950		
Satd. Flow (prot)	0	1845	0	0	1812	1509	1805	1699	0	1736	1671	0
Flt Permitted		0.852			0.964		0.488			0.066		
Satd. Flow (perm)	0	1599	0	0	1754	1509	927	1699	0	121	1671	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		2372			910			1113			1927	
Travel Time (s)		53.9			20.7			25.3			43.8	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	1%	0%	60%	0%	7%	0%	11%	33%	4%	13%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	262	0	0	81	184	6	967	0	275	483	0
Turn Type	Perm	NA		Perm	NA	Perm	pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	5	2		1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0		24.0	24.0	24.0	9.5	24.0		9.5	24.0	
Total Split (s)	24.0	24.0		24.0	24.0	24.0	9.6	60.0		16.0	66.4	
Total Split (%)	24.0%	24.0%		24.0%	24.0%	24.0%	9.6%	60.0%		16.0%	66.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.5	4.0		3.5	4.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	0.0	2.0		0.0	2.0	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		6.0			6.0	6.0	3.5	6.0		3.5	6.0	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Recall Mode	None	None		None	None	None	None	Max		None	Max	
Act Effect Green (s)		17.8			17.8	17.8	62.1	54.0		72.5	68.2	
Actuated g/C Ratio		0.18			0.18	0.18	0.62	0.54		0.73	0.68	

Lanes, Volumes, Timings

4: US 30 & 11th Street

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio		0.92			0.26	0.69	0.01	1.05		0.95	0.42	
Control Delay		78.8			37.9	52.8	4.3	68.7		69.5	9.1	
Queue Delay		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		78.8			37.9	52.8	4.3	68.7		69.5	9.1	
LOS		E			D	D	A	E		E	A	
Approach Delay		78.8			48.2			68.3			31.0	
Approach LOS		E			D			E			C	
Queue Length 50th (ft)		165			45	111	1	~678		125	113	
Queue Length 95th (ft)		#315			88	#200	4	#918		#286	234	
Internal Link Dist (ft)		2292			830			1033			1847	
Turn Bay Length (ft)						190	125			190		
Base Capacity (vph)		288			316	272	635	919		290	1141	
Starvation Cap Reductn		0			0	0	0	0		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		0.91			0.26	0.68	0.01	1.05		0.95	0.42	

Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 99.8

Natural Cycle: 100

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.05

Intersection Signal Delay: 54.6

Intersection LOS: D

Intersection Capacity Utilization 96.4%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 4: US 30 & 11th Street

 Ø1	 Ø2	 Ø4
16 s	60 s	24 s
 Ø5	 Ø6	 Ø8
9.6 s	66.4 s	24 s

HCM 6th TWSC
5: Wolf'S Crossing Road & Hoffman Blvd.

01/21/2021

Intersection

Int Delay, s/veh 0.9

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	4	363	190	15	31	8
Future Vol, veh/h	4	363	190	15	31	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	8	5	0	0	0
Mvmt Flow	4	382	200	16	33	8

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	216	0	0 598 208
Stage 1	-	-	- 208 -
Stage 2	-	-	- 390 -
Critical Hdwy	4.1	-	- 6.4 6.2
Critical Hdwy Stg 1	-	-	- 5.4 -
Critical Hdwy Stg 2	-	-	- 5.4 -
Follow-up Hdwy	2.2	-	- 3.5 3.3
Pot Cap-1 Maneuver	1366	-	- 468 837
Stage 1	-	-	- 832 -
Stage 2	-	-	- 689 -
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	1366	-	- 467 837
Mov Cap-2 Maneuver	-	-	- 467 -
Stage 1	-	-	- 830 -
Stage 2	-	-	- 689 -

Approach	EB	WB	SB
HCM Control Delay, s	0.1	0	12.6
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1366	-	-	-	514
HCM Lane V/C Ratio	0.003	-	-	-	0.08
HCM Control Delay (s)	7.6	-	-	-	12.6
HCM Lane LOS	A	-	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	0.3

Lanes, Volumes, Timings
1: US 30 & Wolf'S Crossing Road

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	17	245	77	93	391	215	143	751	14	130	804	20
Future Volume (vph)	17	245	77	93	391	215	143	751	14	130	804	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	0		0	280		0	130		0	282		0
Storage Lanes	0		0	1		0	1		0	1		0
Taper Length (ft)	25			255			112			180		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.969			0.947			0.997			0.996	
Flt Protected		0.997		0.950			0.950			0.950		
Satd. Flow (prot)	0	1836	0	1805	1763	0	1805	1789	0	1752	1804	0
Flt Permitted		0.405		0.399			0.087			0.087		
Satd. Flow (perm)	0	746	0	758	1763	0	165	1789	0	160	1804	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		1099			2118			1789			736	
Travel Time (s)		25.0			48.1			40.7			16.7	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	1%	4%	0%	6%	0%	3%	5%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	357	0	98	638	0	151	806	0	137	867	0
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0		24.0	24.0		9.5	24.0		9.5	24.0	
Total Split (s)	38.0	38.0		38.0	38.0		10.0	52.0		10.0	52.0	
Total Split (%)	38.0%	38.0%		38.0%	38.0%		10.0%	52.0%		10.0%	52.0%	
Yellow Time (s)	4.0	4.0		4.0	4.0		3.5	4.0		3.5	4.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)		0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		6.0		6.0	6.0		3.5	6.0		3.5	6.0	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Recall Mode	None	None		None	None		None	Max		None	Max	
Act Effect Green (s)		32.0		32.0	32.0		55.0	46.0		55.0	46.0	
Actuated g/C Ratio		0.32		0.32	0.32		0.55	0.46		0.55	0.46	

Lanes, Volumes, Timings
1: US 30 & Wolf'S Crossing Road

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio		1.50		0.40	1.13		0.77	0.98		0.72	1.05	
Control Delay		274.3		32.8	112.7		41.6	54.9		36.8	72.0	
Queue Delay		0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay		274.3		32.8	112.7		41.6	54.9		36.8	72.0	
LOS		F		C	F		D	D		D	E	
Approach Delay		274.3			102.0			52.8			67.2	
Approach LOS		F			F			D			E	
Queue Length 50th (ft)		~317		48	~475		41	489		37	~603	
Queue Length 95th (ft)		#493		99	#688		#141	#754		#124	#837	
Internal Link Dist (ft)		1019			2038			1709			656	
Turn Bay Length (ft)				280			130			282		
Base Capacity (vph)		238		242	564		197	822		191	829	
Starvation Cap Reductn		0		0	0		0	0		0	0	
Spillback Cap Reductn		0		0	0		0	0		0	0	
Storage Cap Reductn		0		0	0		0	0		0	0	
Reduced v/c Ratio		1.50		0.40	1.13		0.77	0.98		0.72	1.05	

Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 100

Natural Cycle: 140

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.50

Intersection Signal Delay: 95.3

Intersection LOS: F

Intersection Capacity Utilization 108.9%

ICU Level of Service G

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: US 30 & Wolf'S Crossing Road

 Ø1	 Ø2	 Ø4
10 s	52 s	38 s
 Ø5	 Ø6	 Ø8
10 s	52 s	38 s

Lanes, Volumes, Timings

2: Eola Road & Wolf'S Crossing Road

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	129	231	1	41	428	94	48	237	39	58	238	164
Future Volume (vph)	129	231	1	41	428	94	48	237	39	58	238	164
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	2000	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	185		0	260		0	165		0	220		0
Storage Lanes	1		0	1		0	1		0	1		1
Taper Length (ft)	140			220			180			170		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.999			0.973			0.979				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1861	0	1626	1804	0	1805	3441	0	1770	1980	1599
Flt Permitted	0.184			0.607			0.506			0.574		
Satd. Flow (perm)	350	1861	0	1039	1804	0	961	3441	0	1069	1980	1599
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		2118			1282			1814			1197	
Travel Time (s)		48.1			29.1			41.2			27.2	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	2%	0%	11%	3%	0%	0%	1%	13%	2%	1%	1%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	136	244	0	43	550	0	51	290	0	61	251	173
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		6
Detector Phase	7	4		3	8		5	2		1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		9.5	24.0		9.5	24.0		9.5	24.0	24.0
Total Split (s)	10.0	36.0		10.0	36.0		10.0	24.0		10.0	24.0	24.0
Total Split (%)	12.5%	45.0%		12.5%	45.0%		12.5%	30.0%		12.5%	30.0%	30.0%
Yellow Time (s)	3.5	4.0		3.5	4.0		3.5	4.0		3.5	4.0	4.0
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	6.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	Yes
Recall Mode	None	None		None	None		None	C-Max		None	C-Max	C-Max
Act Effect Green (s)	38.1	31.7		36.4	27.7		30.7	24.3		30.7	24.3	24.3
Actuated g/C Ratio	0.48	0.40		0.46	0.35		0.38	0.30		0.38	0.30	0.30

Lanes, Volumes, Timings

2: Eola Road & Wolf'S Crossing Road

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.48	0.33		0.08	0.88		0.12	0.28		0.13	0.42	0.36
Control Delay	16.0	18.8		9.7	41.7		16.5	24.4		16.6	27.5	27.4
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	16.0	18.8		9.7	41.7		16.5	24.4		16.6	27.5	27.4
LOS	B	B		A	D		B	C		B	C	C
Approach Delay		17.8			39.3			23.2			26.1	
Approach LOS		B			D			C			C	
Queue Length 50th (ft)	32	86		10	242		16	64		19	112	76
Queue Length 95th (ft)	60	143		24	#412		38	100		43	185	136
Internal Link Dist (ft)		2038			1202			1734			1117	
Turn Bay Length (ft)	185			260			165			220		
Base Capacity (vph)	284	736		524	676		437	1046		467	601	485
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.48	0.33		0.08	0.81		0.12	0.28		0.13	0.42	0.36

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.88

Intersection Signal Delay: 28.2

Intersection LOS: C

Intersection Capacity Utilization 68.1%

ICU Level of Service C

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 2: Eola Road & Wolf'S Crossing Road

				
Ø1	Ø2 (R)		Ø3	Ø4
10 s	24 s		10 s	36 s
				
Ø5	Ø6 (R)		Ø7	Ø8
10 s	24 s		10 s	36 s

Lanes, Volumes, Timings
3: US 30 & Eola Road

01/21/2021

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	267	1	921	238	2	972
Future Volume (vph)	267	1	921	238	2	972
Ideal Flow (vphpl)	1900	1900	2000	1900	1900	2000
Lane Width (ft)	12	12	12	12	12	12
Grade (%)	0%		0%			0%
Storage Length (ft)	310	0		240	240	
Storage Lanes	1	1		1	1	
Taper Length (ft)	200				240	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor						
Frt		0.850		0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1787	1615	1905	1599	1805	1905
Flt Permitted	0.950				0.121	
Satd. Flow (perm)	1787	1615	1905	1599	230	1905
Right Turn on Red		No		No		
Satd. Flow (RTOR)						
Link Speed (mph)	30		30			30
Link Distance (ft)	660		2004			828
Travel Time (s)	15.0		45.5			18.8
Confl. Peds. (#/hr)						
Confl. Bikes (#/hr)						
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	1%	0%	5%	1%	0%	5%
Bus Blockages (#/hr)	0	0	0	0	0	0
Parking (#/hr)						
Mid-Block Traffic (%)	0%		0%			0%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	281	1	969	251	2	1023
Turn Type	Prot	Perm	NA	Perm	pm+pt	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2	6	
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	24.0	24.0	24.0	24.0	9.5	24.0
Total Split (s)	24.0	24.0	56.5	56.5	9.5	66.0
Total Split (%)	26.7%	26.7%	62.8%	62.8%	10.6%	73.3%
Yellow Time (s)	4.0	4.0	4.0	4.0	3.5	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	0.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	3.5	6.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Recall Mode	None	None	C-Max	C-Max	None	C-Max
Act Effect Green (s)	16.9	16.9	59.3	59.3	63.6	61.1
Actuated g/C Ratio	0.19	0.19	0.66	0.66	0.71	0.68

Lanes, Volumes, Timings

3: US 30 & Eola Road

01/21/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
v/c Ratio	0.84	0.00	0.77	0.24	0.01	0.79
Control Delay	57.6	29.0	17.7	7.8	4.5	16.3
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.4
Total Delay	57.6	29.0	17.7	7.8	4.5	16.6
LOS	E	C	B	A	A	B
Approach Delay	57.5		15.7			16.6
Approach LOS	E		B			B
Queue Length 50th (ft)	153	0	329	50	0	369
Queue Length 95th (ft)	#276	5	#747	112	2	567
Internal Link Dist (ft)	580		1924			748
Turn Bay Length (ft)	310			240	240	
Base Capacity (vph)	357	323	1254	1053	267	1293
Starvation Cap Reductn	0	0	0	0	0	46
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.79	0.00	0.77	0.24	0.01	0.82

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.84

Intersection Signal Delay: 20.7

Intersection LOS: C

Intersection Capacity Utilization 73.4%

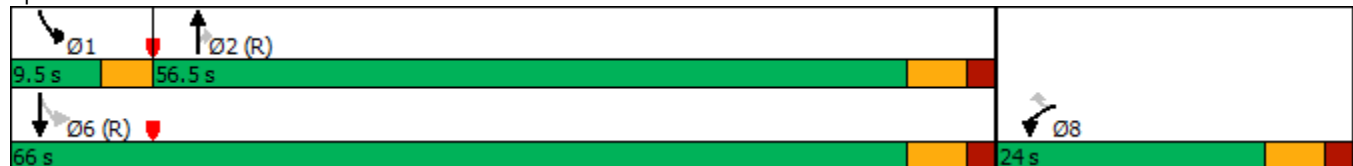
ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: US 30 & Eola Road



Lanes, Volumes, Timings

4: US 30 & 11th Street

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	82	71	7	18	215	317	20	759	10	186	923	130
Future Volume (vph)	82	71	7	18	215	317	20	759	10	186	923	130
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	0		0	0		190	125		0	190		0
Storage Lanes	0		0	0		1	1		0	1		0
Taper Length (ft)	25			25			150			160		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor												
Frt		0.994				0.850		0.998			0.981	
Flt Protected		0.975			0.996		0.950			0.950		
Satd. Flow (prot)	0	1833	0	0	1808	1509	1805	1704	0	1736	1670	0
Flt Permitted		0.428			0.967		0.065			0.187		
Satd. Flow (perm)	0	805	0	0	1756	1509	124	1704	0	342	1670	0
Right Turn on Red			No			No			No			No
Satd. Flow (RTOR)												
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		2372			910			1113			1927	
Travel Time (s)		53.9			20.7			25.3			43.8	
Confl. Peds. (#/hr)												
Confl. Bikes (#/hr)												
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	1%	0%	60%	0%	7%	0%	11%	33%	4%	13%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	168	0	0	245	334	21	810	0	196	1109	0
Turn Type	Perm	NA		Perm	NA	Perm	pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	5	2		1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0		24.0	24.0	24.0	9.5	24.0		9.5	24.0	
Total Split (s)	24.0	24.0		24.0	24.0	24.0	10.0	65.0		11.0	66.0	
Total Split (%)	24.0%	24.0%		24.0%	24.0%	24.0%	10.0%	65.0%		11.0%	66.0%	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.5	4.0		3.5	4.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	0.0	2.0		0.0	2.0	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		6.0			6.0	6.0	3.5	6.0		3.5	6.0	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Recall Mode	None	None		None	None	None	None	Max		None	Max	
Act Effect Green (s)		18.0			18.0	18.0	67.4	59.0		71.5	66.0	
Actuated g/C Ratio		0.18			0.18	0.18	0.67	0.59		0.72	0.66	

Lanes, Volumes, Timings

4: US 30 & 11th Street

01/21/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio		1.17			0.78	1.23	0.12	0.81		0.56	1.00	
Control Delay		165.0			57.0	168.4	5.3	23.9		10.9	47.6	
Queue Delay		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		165.0			57.0	168.4	5.3	23.9		10.9	47.6	
LOS		F			E	F	A	C		B	D	
Approach Delay		165.0			121.2			23.4			42.1	
Approach LOS		F			F			C			D	
Queue Length 50th (ft)		~128			150	~264	3	375		33	552	
Queue Length 95th (ft)		#258			#268	#437	9	564		54	#1056	
Internal Link Dist (ft)		2292			830			1033			1847	
Turn Bay Length (ft)						190	125			190		
Base Capacity (vph)		144			316	271	193	1006		349	1104	
Starvation Cap Reductn		0			0	0	0	0		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		1.17			0.78	1.23	0.11	0.81		0.56	1.00	

Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 99.9

Natural Cycle: 150

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.23

Intersection Signal Delay: 59.8

Intersection LOS: E

Intersection Capacity Utilization 100.0%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 4: US 30 & 11th Street

 Ø1	 Ø2	 Ø4
11 s	65 s	24 s
 Ø5	 Ø6	 Ø8
10 s	66 s	24 s

HCM 6th TWSC
5: Wolf'S Crossing Road & Hoffman Blvd.

01/21/2021

Intersection

Int Delay, s/veh 1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	32	296	544	45	20	19
Future Vol, veh/h	32	296	544	45	20	19
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	8	5	0	0	0
Mvmt Flow	34	312	573	47	21	20

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	620	0	0 977 597
Stage 1	-	-	- 597 -
Stage 2	-	-	- 380 -
Critical Hdwy	4.1	-	- 6.4 6.2
Critical Hdwy Stg 1	-	-	- 5.4 -
Critical Hdwy Stg 2	-	-	- 5.4 -
Follow-up Hdwy	2.2	-	- 3.5 3.3
Pot Cap-1 Maneuver	970	-	- 281 507
Stage 1	-	-	- 554 -
Stage 2	-	-	- 696 -
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	970	-	- 271 507
Mov Cap-2 Maneuver	-	-	- 271 -
Stage 1	-	-	- 535 -
Stage 2	-	-	- 696 -

Approach	EB	WB	SB
HCM Control Delay, s	0.9	0	16.6
HCM LOS			C

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	970	-	-	-	350
HCM Lane V/C Ratio	0.035	-	-	-	0.117
HCM Control Delay (s)	8.8	-	-	-	16.6
HCM Lane LOS	A	-	-	-	C
HCM 95th %tile Q(veh)	0.1	-	-	-	0.4

Capacity Analysis – Year 2036 Total Projected Conditions

Lanes, Volumes, Timings
1: US 30 & Wolf'S Crossing Road

03/18/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	46	420	107	68	242	164	68	962	8	127	811	7
Future Volume (vph)	46	420	107	68	242	164	68	962	8	127	811	7
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	280		0	130		0	282		0
Storage Lanes	0		0	1		0	1		0	1		0
Taper Length (ft)	25			255			112			180		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.975			0.939			0.999			0.999	
Flt Protected		0.996		0.950			0.950			0.950		
Satd. Flow (prot)	0	1805	0	1752	1689	0	1612	1694	0	1656	1725	0
Flt Permitted		0.642		0.223			0.085			0.083		
Satd. Flow (perm)	0	1164	0	411	1689	0	144	1694	0	145	1725	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		12			36			1			1	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		1099			2118			1789			736	
Travel Time (s)		25.0			48.1			40.7			16.7	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	3%	0%	3%	2%	11%	12%	12%	14%	9%	10%	17%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	603	0	72	428	0	72	1021	0	134	861	0
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0		24.0	24.0		9.5	24.0		9.5	24.0	
Total Split (s)	38.0	38.0		38.0	38.0		9.6	52.4		9.6	52.4	
Total Split (%)	38.0%	38.0%		38.0%	38.0%		9.6%	52.4%		9.6%	52.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0		3.5	4.0		3.5	4.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)		0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		6.0		6.0	6.0		3.5	6.0		3.5	6.0	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Recall Mode	None	None		None	None		None	Max		None	Max	
Act Effect Green (s)		32.0		32.0	32.0		54.9	46.4		55.7	48.3	
Actuated g/C Ratio		0.32		0.32	0.32		0.55	0.46		0.56	0.48	
v/c Ratio		1.59		0.55	0.76		0.43	1.30		0.78	1.03	
Control Delay		303.4		46.6	38.0		17.9	170.3		46.6	67.9	
Queue Delay		0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay		303.4		46.6	38.0		17.9	170.3		46.6	67.9	
LOS		F		D	D		B	F		D	E	
Approach Delay		303.4			39.2			160.3			65.0	
Approach LOS		F			D			F			E	
Queue Length 50th (ft)		~551		38	224		19	~843		36	~616	
Queue Length 95th (ft)		#766		#99	#348		40	#1090		#135	#854	

Lanes, Volumes, Timings

1: US 30 & Wolf'S Crossing Road

03/18/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	1019			2038			1709			656		
Turn Bay Length (ft)				280			130			282		
Base Capacity (vph)	380			131			564			168		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	1.59			0.55			0.76			0.43		

Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 100

Natural Cycle: 150

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.59

Intersection Signal Delay: 138.7

Intersection LOS: F

Intersection Capacity Utilization 130.4%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: US 30 & Wolf'S Crossing Road

		
Ø1	Ø2	Ø4
9.6 s	52.4 s	38 s
		
Ø5	Ø6	Ø8
9.6 s	52.4 s	38 s

Lanes, Volumes, Timings

2: Eola Road & Wolf'S Crossing Road

03/18/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	106	254	9	51	175	23	135	305	148	67	151	128
Future Volume (vph)	106	254	9	51	175	23	135	305	148	67	151	128
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	2000	1900
Storage Length (ft)	185		0	260		0	165		0	220		0
Storage Lanes	1		0	1		0	1		0	1		1
Taper Length (ft)	140			220			180			170		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Frt		0.995			0.983			0.951				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1787	1787	0	1570	1777	0	1805	3343	0	1805	1961	1568
Flt Permitted	0.483			0.486			0.608			0.479		
Satd. Flow (perm)	909	1787	0	803	1777	0	1155	3343	0	910	1961	1568
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		2			8			107				136
Link Speed (mph)		30			30			30				30
Link Distance (ft)		2118			1282			1814			1197	
Travel Time (s)		48.1			29.1			41.2			27.2	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	1%	6%	0%	15%	5%	6%	0%	4%	0%	0%	2%	3%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	112	276	0	54	208	0	142	477	0	71	159	135
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		6
Detector Phase	7	4		3	8		5	2		1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		9.5	24.0		9.5	24.0		9.5	24.0	24.0
Total Split (s)	11.0	29.0		10.0	28.0		11.0	31.0		10.0	30.0	30.0
Total Split (%)	13.8%	36.3%		12.5%	35.0%		13.8%	38.8%		12.5%	37.5%	37.5%
Yellow Time (s)	3.5	4.0		3.5	4.0		3.5	4.0		3.5	4.0	4.0
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	6.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	Yes
Recall Mode	None	None		None	None		None	C-Max		None	C-Max	C-Max
Act Effect Green (s)	25.9	18.9		23.8	16.1		43.9	34.8		40.8	31.7	31.7
Actuated g/C Ratio	0.32	0.24		0.30	0.20		0.55	0.44		0.51	0.40	0.40
v/c Ratio	0.30	0.65		0.18	0.57		0.20	0.32		0.13	0.20	0.19
Control Delay	18.2	34.6		16.3	33.1		11.0	14.4		11.1	19.7	4.9
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	18.2	34.6		16.3	33.1		11.0	14.4		11.1	19.7	4.9
LOS	B	C		B	C		B	B		B	B	A
Approach Delay		29.8			29.7			13.6			12.6	
Approach LOS		C			C			B			B	
Queue Length 50th (ft)	37	127		17	90		33	66		16	55	0
Queue Length 95th (ft)	62	189		35	142		72	116		41	108	38

Lanes, Volumes, Timings

2: Eola Road & Wolf'S Crossing Road

03/18/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	2038			1202			1734			1117		
Turn Bay Length (ft)	185			260			165			220		
Base Capacity (vph)	377	515		301	494		701	1513		543	777	703
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.30	0.54		0.18	0.42		0.20	0.32		0.13	0.20	0.19

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 70

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.65

Intersection Signal Delay: 19.8

Intersection LOS: B

Intersection Capacity Utilization 52.1%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 2: Eola Road & Wolf'S Crossing Road

 Ø1	 Ø2 (R)	 Ø3	 Ø4
10 s	31 s	10 s	29 s
 Ø5	 Ø6 (R)	 Ø7	 Ø8
11 s	30 s	11 s	28 s

Lanes, Volumes, Timings

3: US 30 & Eola Road

03/18/2021

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	205	32	1033	285	43	943
Future Volume (vph)	205	32	1033	285	43	943
Ideal Flow (vphpl)	1900	1900	2000	1900	1900	2000
Storage Length (ft)	0	0		240	240	
Storage Lanes	1	1		1	1	
Taper Length (ft)	200				240	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850		0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1736	1615	1770	1583	1805	1802
Flt Permitted	0.950				0.068	
Satd. Flow (perm)	1736	1615	1770	1583	129	1802
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		34		263		
Link Speed (mph)	30		30			30
Link Distance (ft)	1415		2004			828
Travel Time (s)	32.2		45.5			18.8
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	4%	0%	13%	2%	0%	11%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	216	34	1087	300	45	993
Turn Type	Prot	Perm	NA	Perm	pm+pt	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2	6	
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	24.0	24.0	24.0	24.0	9.5	24.0
Total Split (s)	24.0	24.0	56.0	56.0	10.0	66.0
Total Split (%)	26.7%	26.7%	62.2%	62.2%	11.1%	73.3%
Yellow Time (s)	4.0	4.0	4.0	4.0	3.5	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	0.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	3.5	6.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Recall Mode	None	None	C-Max	C-Max	None	C-Max
Act Effect Green (s)	15.3	15.3	56.8	56.8	65.2	62.7
Actuated g/C Ratio	0.17	0.17	0.63	0.63	0.72	0.70
v/c Ratio	0.73	0.11	0.97	0.27	0.22	0.79
Control Delay	46.7	13.3	41.5	2.6	6.6	16.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	46.7	13.3	41.5	2.6	6.6	16.2
LOS	D	B	D	A	A	B
Approach Delay	42.1		33.1			15.8
Approach LOS	D		C			B
Queue Length 50th (ft)	98	0	~687	8	6	343
Queue Length 95th (ft)	155	22	#951	45	17	583

Lanes, Volumes, Timings

3: US 30 & Eola Road

03/18/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Internal Link Dist (ft)	1335		1924			748
Turn Bay Length (ft)				240	240	
Base Capacity (vph)	347	350	1116	1095	214	1255
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	0.62	0.10	0.97	0.27	0.21	0.79

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 100

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.97

Intersection Signal Delay: 27.2

Intersection LOS: C

Intersection Capacity Utilization 73.0%

ICU Level of Service D

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: US 30 & Eola Road



Lanes, Volumes, Timings
4: US 30 & 111th Street

03/18/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	95	158	10	40	79	228	6	939	44	311	785	67
Future Volume (vph)	95	158	10	40	79	228	6	939	44	311	785	67
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		190	125		0	190		0
Storage Lanes	0		0	0		1	1		0	1		0
Taper Length (ft)	25			25			150			160		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.995				0.850		0.993			0.988	
Flt Protected		0.982			0.983		0.950			0.950		
Satd. Flow (prot)	0	1845	0	0	1554	1509	1805	1685	0	1736	1674	0
Flt Permitted		0.814			0.699		0.232			0.066		
Satd. Flow (perm)	0	1530	0	0	1105	1509	441	1685	0	121	1674	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		2				238		4			8	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		2372			910			1113			1927	
Travel Time (s)		53.9			20.7			25.3			43.8	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	1%	0%	60%	0%	7%	0%	11%	33%	4%	13%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	277	0	0	125	240	6	1034	0	327	897	0
Turn Type	Perm	NA		Perm	NA	Perm	pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	5	2		1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0		24.0	24.0	24.0	9.5	24.0		9.5	24.0	
Total Split (s)	24.0	24.0		24.0	24.0	24.0	9.6	60.0		16.0	66.4	
Total Split (%)	24.0%	24.0%		24.0%	24.0%	24.0%	9.6%	60.0%		16.0%	66.4%	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.5	4.0		3.5	4.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	0.0	2.0		0.0	2.0	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		6.0			6.0	6.0	3.5	6.0		3.5	6.0	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Recall Mode	None	None		None	None	None	None	Max		None	Max	
Act Effect Green (s)		18.0			18.0	18.0	62.1	54.0		72.5	68.1	
Actuated g/C Ratio		0.18			0.18	0.18	0.62	0.54		0.72	0.68	
v/c Ratio		1.00			0.63	0.52	0.02	1.14		1.13	0.78	
Control Delay		96.6			53.6	9.3	4.5	98.7		121.8	18.2	
Queue Delay		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		96.6			53.6	9.3	4.5	98.7		121.8	18.2	
LOS		F			D	A	A	F		F	B	
Approach Delay		96.6			24.5			98.2			45.9	
Approach LOS		F			C			F			D	
Queue Length 50th (ft)		177			74	1	1	~773		~194	319	
Queue Length 95th (ft)		#347			#150	67	4	#1021		#368	#752	

Lanes, Volumes, Timings

4: US 30 & 111th Street

03/18/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		2292			830			1033			1847	
Turn Bay Length (ft)						190	125			190		
Base Capacity (vph)		277			198	466	359	911		289	1143	
Starvation Cap Reductn		0			0	0	0	0		0	0	
Spillback Cap Reductn		0			0	0	0	0		0	0	
Storage Cap Reductn		0			0	0	0	0		0	0	
Reduced v/c Ratio		1.00			0.63	0.52	0.02	1.14		1.13	0.78	

Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 100

Natural Cycle: 110

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.14

Intersection Signal Delay: 66.8

Intersection LOS: E

Intersection Capacity Utilization 103.5%

ICU Level of Service G

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 4: US 30 & 111th Street

 Ø1	 Ø2	 Ø4
16 s	60 s	24 s
 Ø5	 Ø6	 Ø8
9.6 s	66.4 s	24 s

Lanes, Volumes, Timings

9: Eola Road & Commercial Full Access/Lincoln Crossing Full Access

03/18/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	17	1	29	70	1	114	32	267	29	33	138	28
Future Volume (vph)	17	1	29	70	1	114	32	267	29	33	138	28
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	2000	1900	1900	2000	1900
Storage Length (ft)	0		0	0		0	185		115	115		185
Storage Lanes	1		0	1		0	1		1	1		1
Taper Length (ft)	25			25			200			156		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.855			0.851				0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1594	0	1805	1617	0	1770	3725	1615	1805	3725	1583
Flt Permitted	0.784			0.465			0.660			0.579		
Satd. Flow (perm)	1460	1594	0	884	1617	0	1229	3725	1615	1100	3725	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		31			120				79			121
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		403			295			1415			1814	
Travel Time (s)		9.2			6.7			32.2			41.2	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	0%	2%	0%	0%	0%	2%	2%	0%	0%	2%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	18	32	0	74	121	0	34	281	31	35	145	29
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA	pm+ov	pm+pt	NA	pm+ov
Protected Phases	7	4		3	8		5	2	3	1	6	7
Permitted Phases	4			8			2		2	6		6
Detector Phase	7	4		3	8		5	2	3	1	6	7
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		24.0	24.0		9.5	24.0	24.0	9.5	24.0	9.5
Total Split (s)	10.0	25.0		24.0	39.0		10.0	31.0	24.0	10.0	31.0	10.0
Total Split (%)	11.1%	27.8%		26.7%	43.3%		11.1%	34.4%	26.7%	11.1%	34.4%	11.1%
Yellow Time (s)	3.5	4.0		3.5	4.0		3.5	4.0	3.5	3.5	4.0	3.5
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0	0.0	0.0	2.0	0.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0	3.5	3.5	6.0	3.5
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag	Lead	Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None		None	C-Max	None	None	C-Max	None
Act Effect Green (s)	12.1	6.2		16.5	8.3		64.1	57.8	73.1	64.1	57.8	69.8
Actuated g/C Ratio	0.13	0.07		0.18	0.09		0.71	0.64	0.81	0.71	0.64	0.78
v/c Ratio	0.08	0.23		0.30	0.47		0.04	0.12	0.02	0.04	0.06	0.02
Control Delay	26.7	18.9		31.0	13.6		7.5	11.3	0.0	5.2	8.9	0.0
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.7	18.9		31.0	13.6		7.5	11.3	0.0	5.2	8.9	0.0
LOS	C	B		C	B		A	B	A	A	A	A
Approach Delay		21.7			20.2			9.9			7.0	
Approach LOS		C			C			A			A	
Queue Length 50th (ft)	8	1		34	1		6	40	0	5	18	0
Queue Length 95th (ft)	24	28		65	48		24	85	0	17	37	0

Lanes, Volumes, Timings

9: Eola Road & Commercial Full Access/Lincoln Crossing Full Access

03/18/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	323			215			1335			1734		
Turn Bay Length (ft)							185				115	185
Base Capacity (vph)	222	360		424	668		917	2390	1455	837	2390	1262
Starvation Cap Reductn	0	0		0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.08	0.09		0.17	0.18		0.04	0.12	0.02	0.04	0.06	0.02

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 85

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.47

Intersection Signal Delay: 12.4

Intersection LOS: B

Intersection Capacity Utilization 35.1%

ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 9: Eola Road & Commercial Full Access/Lincoln Crossing Full Access

 Ø1	 Ø2 (R)	 Ø3	 Ø4
10 s	31 s	24 s	25 s
 Ø5	 Ø6 (R)	 Ø7	 Ø8
10 s	31 s	10 s	39 s

HCM 6th TWSC
5: Wolf'S Crossing Road & Hoffman Blvd.

03/18/2021

Intersection

Int Delay, s/veh 0.8

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	4	469	231	15	31	8
Future Vol, veh/h	4	469	231	15	31	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	8	5	0	0	0
Mvmt Flow	4	494	243	16	33	8

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	259	0	0
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	4.1	-	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	2.2	-	-
Pot Cap-1 Maneuver	1317	-	-
Stage 1	-	-	-
Stage 2	-	-	-
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	1317	-	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	SB
HCM Control Delay, s	0.1	0	14.4
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1317	-	-	-	424
HCM Lane V/C Ratio	0.003	-	-	-	0.097
HCM Control Delay (s)	7.7	-	-	-	14.4
HCM Lane LOS	A	-	-	-	B
HCM 95th %tile Q(veh)	0	-	-	-	0.3

Intersection

Int Delay, s/veh 0.6

Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	459	10	5	234	15	14
Future Vol, veh/h	459	10	5	234	15	14
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	100	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	0	0	2	0	0
Mvmt Flow	483	11	5	246	16	15

Major/Minor	Major1	Major2	Minor1
Conflicting Flow All	0	0	494
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	-	4.1
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	-	2.2
Pot Cap-1 Maneuver	-	-	1080
Stage 1	-	-	-
Stage 2	-	-	-
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	-	1080
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	EB	WB	NB
HCM Control Delay, s	0	0.2	13.4
HCM LOS			B

Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	458	-	-	1080	-
HCM Lane V/C Ratio	0.067	-	-	0.005	-
HCM Control Delay (s)	13.4	-	-	8.3	-
HCM Lane LOS	B	-	-	A	-
HCM 95th %tile Q(veh)	0.2	-	-	0	-

HCM 6th TWSC
7: US 30 & Commercial RI/RO

03/18/2021

Intersection

Int Delay, s/veh 0.2

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	20	1018	47	0	986
Future Vol, veh/h	0	20	1018	47	0	986
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	240	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	0	13	0	0	11
Mvmt Flow	0	21	1072	49	0	1038

Major/Minor	Minor1	Major1	Major2
Conflicting Flow All	-	1072	0
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	6.2	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	3.3	-
Pot Cap-1 Maneuver	0	270	-
Stage 1	0	-	-
Stage 2	0	-	-
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	-	270	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	19.5	0	0
HCM LOS	C		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBT
Capacity (veh/h)	-	-	270
HCM Lane V/C Ratio	-	-	0.078
HCM Control Delay (s)	-	-	19.5
HCM Lane LOS	-	-	C
HCM 95th %tile Q(veh)	-	-	0.3

HCM 6th TWSC
8: US 30 & Lincoln Prairie Full Access

03/18/2021

Intersection

Int Delay, s/veh 0.9

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	28	32	1240	22	24	1124
Future Vol, veh/h	28	32	1240	22	24	1124
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	240	240	-
Veh in Median Storage, #	1	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	13	0	0	11
Mvmt Flow	29	34	1305	23	25	1183

Major/Minor	Minor1	Major1	Major2
Conflicting Flow All	2538	1305	0
Stage 1	1305	-	-
Stage 2	1233	-	-
Critical Hdwy	6.4	6.2	-
Critical Hdwy Stg 1	5.4	-	-
Critical Hdwy Stg 2	5.4	-	-
Follow-up Hdwy	3.5	3.3	-
Pot Cap-1 Maneuver	30	198	-
Stage 1	256	-	-
Stage 2	278	-	-
Platoon blocked, %		-	-
Mov Cap-1 Maneuver	~ 29	198	-
Mov Cap-2 Maneuver	132	-	-
Stage 1	256	-	-
Stage 2	265	-	-

Approach	WB	NB	SB
HCM Control Delay, s	33	0	0.3
HCM LOS	D		

Minor Lane/Major Mvmt	NBT	NBRWBLn1WBLn2	SBL	SBT
Capacity (veh/h)	-	- 132	198	527
HCM Lane V/C Ratio	-	- 0.223	0.17	0.048
HCM Control Delay (s)	-	- 39.9	26.9	12.2
HCM Lane LOS	-	- E	D	B
HCM 95th %tile Q(veh)	-	- 0.8	0.6	0.2

Notes

~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon

HCM 6th TWSC
11: 111th Street & Full Access Drive

03/18/2021

Intersection

Int Delay, s/veh 2.1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	54	459	290	32	42	57
Future Vol, veh/h	54	459	290	32	42	57
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	185	-	-	185	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	2	2	0	0	0
Mvmt Flow	57	483	305	34	44	60

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	339	0	0 902 305
Stage 1	-	-	- 305 -
Stage 2	-	-	- 597 -
Critical Hdwy	4.1	-	- 6.4 6.2
Critical Hdwy Stg 1	-	-	- 5.4 -
Critical Hdwy Stg 2	-	-	- 5.4 -
Follow-up Hdwy	2.2	-	- 3.5 3.3
Pot Cap-1 Maneuver	1231	-	- 311 740
Stage 1	-	-	- 752 -
Stage 2	-	-	- 554 -
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	1231	-	- 297 740
Mov Cap-2 Maneuver	-	-	- 297 -
Stage 1	-	-	- 717 -
Stage 2	-	-	- 554 -

Approach	EB	WB	SB
HCM Control Delay, s	0.8	0	15.3
HCM LOS			C

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1231	-	-	-	453
HCM Lane V/C Ratio	0.046	-	-	-	0.23
HCM Control Delay (s)	8.1	-	-	-	15.3
HCM Lane LOS	A	-	-	-	C
HCM 95th %tile Q(veh)	0.1	-	-	-	0.9

Lanes, Volumes, Timings
1: US 30 & Wolf'S Crossing Road

03/19/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	46	420	107	68	242	164	68	962	8	127	811	7
Future Volume (vph)	46	420	107	68	242	164	68	962	8	127	811	7
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		215	280		280	130		0	282		0
Storage Lanes	0		1	1		1	1		0	1		0
Taper Length (ft)	25			255			112			180		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850		0.999			0.999	
Flt Protected		0.995		0.950			0.950			0.950		
Satd. Flow (prot)	0	1841	1615	1752	1961	1455	1612	1694	0	1656	1725	0
Flt Permitted		0.936		0.123			0.068			0.067		
Satd. Flow (perm)	0	1732	1615	227	1961	1455	115	1694	0	117	1725	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			102			70						
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		1099			2118			1789			736	
Travel Time (s)		25.0			48.1			40.7			16.7	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	3%	0%	3%	2%	11%	12%	12%	14%	9%	10%	17%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	490	113	72	255	173	72	1021	0	134	861	0
Turn Type	Perm	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA		pm+pt	NA	
Protected Phases		4	5	3	8	1	5	2		1	6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	4	4	5	3	8	1	5	2		1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0	9.5	9.5	24.0	9.5	9.5	24.0		9.5	24.0	
Total Split (s)	35.0	35.0	10.0	10.0	45.0	10.0	10.0	65.0		10.0	65.0	
Total Split (%)	29.2%	29.2%	8.3%	8.3%	37.5%	8.3%	8.3%	54.2%		8.3%	54.2%	
Yellow Time (s)	4.0	4.0	3.5	3.5	4.0	3.5	3.5	4.0		3.5	4.0	
All-Red Time (s)	2.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0		0.0	2.0	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		6.0	3.5	3.5	6.0	3.5	3.5	6.0		3.5	6.0	
Lead/Lag	Lag	Lag	Lead	Lead		Lead	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes		Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	None	None	None	None	None	None	Max		None	Max	
Act Effect Green (s)		29.0	41.4	39.4	36.9	49.4	67.9	59.1		68.3	59.2	
Actuated g/C Ratio		0.25	0.35	0.33	0.31	0.42	0.58	0.50		0.58	0.50	
v/c Ratio		1.15	0.18	0.46	0.42	0.27	0.49	1.21		0.88	0.99	
Control Delay		132.7	7.3	36.3	34.3	14.0	25.1	132.4		71.1	59.8	
Queue Delay		0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		132.7	7.3	36.3	34.3	14.0	25.1	132.4		71.1	59.8	
LOS		F	A	D	C	B	C	F		E	E	
Approach Delay		109.2			27.5			125.3			61.3	
Approach LOS		F			C			F			E	
Queue Length 50th (ft)		~453	6	38	153	48	22	~974		55	~678	
Queue Length 95th (ft)		#662	46	73	229	99	55	#1228		#176	#949	

Lanes, Volumes, Timings

1: US 30 & Wolf'S Crossing Road

03/19/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	1019			2038			1709			656		
Turn Bay Length (ft)	215			280			130			282		
Base Capacity (vph)	426			634			160			649		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	1.15			0.18			0.45			0.39		

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 118

Natural Cycle: 150

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.21

Intersection Signal Delay: 87.0

Intersection LOS: F

Intersection Capacity Utilization 113.2%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: US 30 & Wolf'S Crossing Road

								
10 s	65 s		10 s	35 s		10 s	45 s	
								
10 s	65 s							

Lanes, Volumes, Timings
4: US 30 & 11th Street

03/19/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	95	158	10	40	79	228	6	939	44	311	785	67
Future Volume (vph)	95	158	10	40	79	228	6	939	44	311	785	67
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	1900	1900	1900	2000	1900
Storage Length (ft)	240		0	240		240	125		0	190		265
Storage Lanes	1		0	1		1	1		0	1		1
Taper Length (ft)	240			240			150			160		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.991				0.850		0.993				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1865	0	1128	2000	1509	1805	1685	0	1736	1770	1583
Flt Permitted	0.581			0.514			0.277			0.059		
Satd. Flow (perm)	1104	1865	0	610	2000	1509	526	1685	0	108	1770	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		2				121		3				71
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		2372			910			1113			1927	
Travel Time (s)		53.9			20.7			25.3			43.8	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	1%	0%	60%	0%	7%	0%	11%	33%	4%	13%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	100	177	0	42	83	240	6	1034	0	327	826	71
Turn Type	pm+pt	NA		pm+pt	NA	pm+ov	pm+pt	NA		pm+pt	NA	pm+ov
Protected Phases	7	4		3	8	1	5	2		1	6	7
Permitted Phases	4			8		8	2			6		6
Detector Phase	7	4		3	8	1	5	2		1	6	7
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		9.5	24.0	9.5	9.5	24.0		9.5	24.0	9.5
Total Split (s)	10.0	24.0		10.0	24.0	19.0	10.0	67.0		19.0	76.0	10.0
Total Split (%)	8.3%	20.0%		8.3%	20.0%	15.8%	8.3%	55.8%		15.8%	63.3%	8.3%
Yellow Time (s)	3.5	4.0		3.5	4.0	3.5	3.5	4.0		3.5	4.0	3.5
All-Red Time (s)	0.0	2.0		0.0	2.0	0.0	0.0	2.0		0.0	2.0	0.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	3.5	6.0		3.5	6.0	3.5	3.5	6.0		3.5	6.0	3.5
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	None		None	None	None	None	Max		None	Max	None
Act Effect Green (s)	22.6	15.0		21.1	14.3	33.0	69.4	61.3		82.8	78.5	93.2
Actuated g/C Ratio	0.20	0.13		0.18	0.12	0.29	0.60	0.53		0.72	0.68	0.81
v/c Ratio	0.38	0.72		0.30	0.34	0.46	0.02	1.15		1.10	0.68	0.05
Control Delay	41.0	65.2		41.4	49.8	18.2	6.7	109.2		115.8	16.8	1.3
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	41.0	65.2		41.4	49.8	18.2	6.7	109.2		115.8	16.8	1.3
LOS	D	E		D	D	B	A	F		F	B	A
Approach Delay		56.5			28.1			108.6			42.4	
Approach LOS		E			C			F			D	
Queue Length 50th (ft)	61	129		25	58	67	1	~954		~241	348	0
Queue Length 95th (ft)	110	206		57	107	141	6	#1238		#436	658	14

Lanes, Volumes, Timings

4: US 30 & 11th Street

03/19/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	2292			830			1033			1847		
Turn Bay Length (ft)	240			240		240	125			190		265
Base Capacity (vph)	264	294		142	314	519	393	897		297	1206	1294
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.38	0.60		0.30	0.26	0.46	0.02	1.15		1.10	0.68	0.05

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 115.2

Natural Cycle: 150

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.15

Intersection Signal Delay: 65.6

Intersection LOS: E

Intersection Capacity Utilization 99.1%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 4: US 30 & 11th Street

 Ø1	 Ø2	 Ø3	 Ø4
19 s	67 s	10 s	24 s
 Ø5	 Ø6	 Ø7	 Ø8
10 s	76 s	10 s	24 s

Lanes, Volumes, Timings
1: US 30 & Wolf's Crossing Road

03/18/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↔		↔	↔		↔	↔		↔	↔	
Traffic Volume (vph)	17	254	115	93	395	231	179	938	14	160	1010	20
Future Volume (vph)	17	254	115	93	395	231	179	938	14	160	1010	20
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	280		0	130		0	282		0
Storage Lanes	0		0	1		0	1		0	1		0
Taper Length (ft)	25			255			112			180		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.960			0.945			0.998			0.997	
Flt Protected		0.998		0.950			0.950			0.950		
Satd. Flow (prot)	0	1820	0	1805	1758	0	1805	1790	0	1752	1806	0
Flt Permitted		0.386		0.353			0.087			0.087		
Satd. Flow (perm)	0	704	0	671	1758	0	165	1790	0	160	1806	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		22			31			1			1	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		1099			2118			1789			736	
Travel Time (s)		25.0			48.1			40.7			16.7	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	0%	0%	0%	1%	4%	0%	6%	0%	3%	5%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	406	0	98	659	0	188	1002	0	168	1084	0
Turn Type	Perm	NA		Perm	NA		pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8			2			6		
Detector Phase	4	4		8	8		5	2		1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0		24.0	24.0		9.5	24.0		9.5	24.0	
Total Split (s)	38.0	38.0		38.0	38.0		10.0	52.0		10.0	52.0	
Total Split (%)	38.0%	38.0%		38.0%	38.0%		10.0%	52.0%		10.0%	52.0%	
Yellow Time (s)	4.0	4.0		4.0	4.0		3.5	4.0		3.5	4.0	
All-Red Time (s)	2.0	2.0		2.0	2.0		0.0	2.0		0.0	2.0	
Lost Time Adjust (s)		0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Lost Time (s)		6.0		6.0	6.0		3.5	6.0		3.5	6.0	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Recall Mode	None	None		None	None		None	Max		None	Max	
Act Effct Green (s)		32.0		32.0	32.0		55.0	46.0		55.0	46.0	
Actuated g/C Ratio		0.32		0.32	0.32		0.55	0.46		0.55	0.46	
v/c Ratio		1.69		0.46	1.13		0.95	1.22		0.88	1.30	
Control Delay		353.9		35.5	110.4		75.1	136.0		60.1	172.3	
Queue Delay		0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Total Delay		353.9		35.5	110.4		75.1	136.0		60.1	172.3	
LOS		F		D	F		E	F		E	F	
Approach Delay		353.9			100.7			126.4			157.2	
Approach LOS		F			F			F			F	
Queue Length 50th (ft)		~376		49	~477		66	~792		53	~898	
Queue Length 95th (ft)		#565		103	#698		#204	#1038		#175	#1149	

Lanes, Volumes, Timings

1: US 30 & Wolf's Crossing Road

03/18/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	1019			2038			1709			656		
Turn Bay Length (ft)				280			130			282		
Base Capacity (vph)	240			214			197			191		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	1.69			0.46			0.95			0.88		

Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 100

Natural Cycle: 150

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.69

Intersection Signal Delay: 157.3

Intersection LOS: F

Intersection Capacity Utilization 124.2%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: US 30 & Wolf's Crossing Road

		
Ø1	Ø2	Ø4
10 s	52 s	38 s
		
Ø5	Ø6	Ø8
10 s	52 s	38 s

Lanes, Volumes, Timings

2: Eola Road & Wolf's Crossing Road

03/18/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	129	261	10	148	436	96	60	374	126	61	330	164
Future Volume (vph)	129	261	10	148	436	96	60	374	126	61	330	164
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	2000	1900
Storage Length (ft)	185		0	260		0	165		0	220		0
Storage Lanes	1		0	1		0	1		0	1		1
Taper Length (ft)	140			220			180			170		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	0.95	1.00	1.00	1.00
Frt		0.994			0.973			0.962				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1853	0	1626	1804	0	1805	3338	0	1770	1980	1599
Flt Permitted	0.167			0.503			0.348			0.358		
Satd. Flow (perm)	317	1853	0	861	1804	0	661	3338	0	667	1980	1599
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		3			16			54				173
Link Speed (mph)		30			30			30				30
Link Distance (ft)		2118			1282			1814			1197	
Travel Time (s)		48.1			29.1			41.2			27.2	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	2%	0%	11%	3%	0%	0%	1%	13%	2%	1%	1%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	136	286	0	156	560	0	63	527	0	64	347	173
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA		pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4			8			2			6		6
Detector Phase	7	4		3	8		5	2		1	6	6
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		9.5	24.0		9.5	24.0		9.5	24.0	24.0
Total Split (s)	10.0	36.0		10.0	36.0		10.0	24.0		10.0	24.0	24.0
Total Split (%)	12.5%	45.0%		12.5%	45.0%		12.5%	30.0%		12.5%	30.0%	30.0%
Yellow Time (s)	3.5	4.0		3.5	4.0		3.5	4.0		3.5	4.0	4.0
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0		0.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0		3.5	6.0	6.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag		Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes		Yes	Yes	Yes
Recall Mode	None	None		None	None		None	C-Max		None	C-Max	C-Max
Act Effect Green (s)	36.8	27.8		36.8	27.8		29.9	22.3		29.9	22.3	22.3
Actuated g/C Ratio	0.46	0.35		0.46	0.35		0.37	0.28		0.37	0.28	0.28
v/c Ratio	0.51	0.44		0.34	0.88		0.19	0.54		0.19	0.63	0.30
Control Delay	17.2	21.8		12.7	40.5		17.3	26.0		17.4	34.1	6.1
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	0.0
Total Delay	17.2	21.8		12.7	40.5		17.3	26.0		17.4	34.1	6.1
LOS	B	C		B	D		B	C		B	C	A
Approach Delay		20.3			34.4			25.1			24.0	
Approach LOS		C			C			C			C	
Queue Length 50th (ft)	32	102		38	240		20	113		20	164	0
Queue Length 95th (ft)	60	168		70	#414		44	166		45	#291	47

Lanes, Volumes, Timings

2: Eola Road & Wolf's Crossing Road

03/18/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	2038			1202			1734			1117		
Turn Bay Length (ft)	185			260			165			220		
Base Capacity (vph)	266	696		457	686		340	967		339	550	569
Starvation Cap Reductn	0	0		0	0		0	0		0	0	0
Spillback Cap Reductn	0	0		0	0		0	0		0	0	0
Storage Cap Reductn	0	0		0	0		0	0		0	0	0
Reduced v/c Ratio	0.51	0.41		0.34	0.82		0.19	0.54		0.19	0.63	0.30

Intersection Summary

Area Type: Other

Cycle Length: 80

Actuated Cycle Length: 80

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 75

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.88

Intersection Signal Delay: 26.8

Intersection LOS: C

Intersection Capacity Utilization 73.3%

ICU Level of Service D

Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 2: Eola Road & Wolf's Crossing Road

				
Ø1	Ø2 (R)		Ø3	Ø4
10 s	24 s		10 s	36 s
				
Ø5	Ø6 (R)		Ø7	Ø8
10 s	24 s		10 s	36 s

Lanes, Volumes, Timings

3: US 30 & Eola Road

03/18/2021

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	445	23	1148	313	133	1085
Future Volume (vph)	445	23	1148	313	133	1085
Ideal Flow (vphpl)	1900	1900	2000	1900	1900	2000
Storage Length (ft)	0	0		240	240	
Storage Lanes	1	1		1	1	
Taper Length (ft)	200				240	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850		0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1787	1615	1905	1599	1805	1905
Flt Permitted	0.950				0.075	
Satd. Flow (perm)	1787	1615	1905	1599	142	1905
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		24		260		
Link Speed (mph)	30		30			30
Link Distance (ft)	601		2004			828
Travel Time (s)	13.7		45.5			18.8
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	1%	0%	5%	1%	0%	5%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	468	24	1208	329	140	1142
Turn Type	Prot	Perm	NA	Perm	pm+pt	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2	6	
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	24.0	24.0	24.0	24.0	9.5	24.0
Total Split (s)	24.0	24.0	56.0	56.0	10.0	66.0
Total Split (%)	26.7%	26.7%	62.2%	62.2%	11.1%	73.3%
Yellow Time (s)	4.0	4.0	4.0	4.0	3.5	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	0.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	3.5	6.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Recall Mode	None	None	C-Max	C-Max	None	C-Max
Act Effect Green (s)	18.0	18.0	50.0	50.0	62.5	60.0
Actuated g/C Ratio	0.20	0.20	0.56	0.56	0.69	0.67
v/c Ratio	1.31	0.07	1.14	0.33	0.64	0.90
Control Delay	192.5	18.0	97.5	3.5	25.8	24.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	192.5	18.0	97.5	3.5	25.8	24.2
LOS	F	B	F	A	C	C
Approach Delay	184.0		77.3			24.4
Approach LOS	F		E			C
Queue Length 50th (ft)	~361	0	~811	17	23	477
Queue Length 95th (ft)	#555	25	#1056	56	#98	#849

Lanes, Volumes, Timings

3: US 30 & Eola Road

03/18/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Internal Link Dist (ft)	521		1924			748
Turn Bay Length (ft)				240	240	
Base Capacity (vph)	357	342	1058	1003	218	1270
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.31	0.07	1.14	0.33	0.64	0.90

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 140

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.31

Intersection Signal Delay: 72.7

Intersection LOS: E

Intersection Capacity Utilization 102.8%

ICU Level of Service G

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: US 30 & Eola Road



Lanes, Volumes, Timings
4: US 30 & 111th Street

03/18/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	112	88	7	79	230	499	20	880	78	258	1026	156
Future Volume (vph)	112	88	7	79	230	499	20	880	78	258	1026	156
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		190	125		0	190		0
Storage Lanes	0		0	0		1	1		0	1		0
Taper Length (ft)	25			25			150			160		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.996				0.850		0.988			0.980	
Flt Protected		0.974			0.987		0.950			0.950		
Satd. Flow (prot)	0	1835	0	0	1626	1509	1805	1664	0	1736	1669	0
Flt Permitted		0.202			0.835		0.065			0.069		
Satd. Flow (perm)	0	381	0	0	1376	1509	124	1664	0	126	1669	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		1				232		8			14	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		2372			910			1113			1927	
Travel Time (s)		53.9			20.7			25.3			43.8	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	1%	0%	60%	0%	7%	0%	11%	33%	4%	13%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	218	0	0	325	525	21	1008	0	272	1244	0
Turn Type	Perm	NA		Perm	NA	Perm	pm+pt	NA		pm+pt	NA	
Protected Phases		4			8		5	2		1	6	
Permitted Phases	4			8		8	2			6		
Detector Phase	4	4		8	8	8	5	2		1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0		24.0	24.0	24.0	9.5	24.0		9.5	24.0	
Total Split (s)	24.0	24.0		24.0	24.0	24.0	10.0	65.0		11.0	66.0	
Total Split (%)	24.0%	24.0%		24.0%	24.0%	24.0%	10.0%	65.0%		11.0%	66.0%	
Yellow Time (s)	4.0	4.0		4.0	4.0	4.0	3.5	4.0		3.5	4.0	
All-Red Time (s)	2.0	2.0		2.0	2.0	2.0	0.0	2.0		0.0	2.0	
Lost Time Adjust (s)		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		6.0			6.0	6.0	3.5	6.0		3.5	6.0	
Lead/Lag							Lead	Lag		Lead	Lag	
Lead-Lag Optimize?							Yes	Yes		Yes	Yes	
Recall Mode	None	None		None	None	None	None	Max		None	Max	
Act Effect Green (s)		18.0			18.0	18.0	67.3	59.0		71.6	66.1	
Actuated g/C Ratio		0.18			0.18	0.18	0.67	0.59		0.72	0.66	
v/c Ratio		3.16			1.32	1.14	0.12	1.02		1.29	1.12	
Control Delay		1025.5			202.8	108.5	5.3	56.7		185.5	87.1	
Queue Delay		0.0			0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		1025.5			202.8	108.5	5.3	56.7		185.5	87.1	
LOS		F			F	F	A	E		F	F	
Approach Delay		1025.5			144.5			55.6			104.7	
Approach LOS		F			F			E			F	
Queue Length 50th (ft)		~248			~268	~262	3	~686		~166	~877	
Queue Length 95th (ft)		#398			#439	#468	9	#932		#334	#1245	

Lanes, Volumes, Timings

4: US 30 & 111th Street

03/18/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	2292			830			1033			1847		
Turn Bay Length (ft)							190	125		190		
Base Capacity (vph)	69			247			461	193	985	211	1108	
Starvation Cap Reductn	0			0			0	0	0	0	0	
Spillback Cap Reductn	0			0			0	0	0	0	0	
Storage Cap Reductn	0			0			0	0	0	0	0	
Reduced v/c Ratio	3.16			1.32			1.14	0.11	1.02	1.29	1.12	

Intersection Summary

Area Type: Other

Cycle Length: 100

Actuated Cycle Length: 100

Natural Cycle: 150

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 3.16

Intersection Signal Delay: 155.7

Intersection LOS: F

Intersection Capacity Utilization 113.7%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 4: US 30 & 111th Street

 Ø1	 Ø2	 Ø4
11 s	65 s	24 s
 Ø5	 Ø6	 Ø8
10 s	66 s	24 s

Lanes, Volumes, Timings

9: Eola Road & Commercial Full Access/Lincoln Crossing Full Access

03/18/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	144	1	100	54	1	56	96	289	77	81	314	81
Future Volume (vph)	144	1	100	54	1	56	96	289	77	81	314	81
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	185		115	115		185
Storage Lanes	1		0	1		0	1		1	1		1
Taper Length (ft)	25			25			200			156		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1.00	1.00	0.95	1.00
Frt		0.851			0.852				0.850			0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1770	1585	0	1805	1619	0	1770	3539	1615	1805	3539	1583
Flt Permitted	0.449			0.741			0.549			0.567		
Satd. Flow (perm)	836	1585	0	1408	1619	0	1023	3539	1615	1077	3539	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		105			59				81			121
Link Speed (mph)		25			25			30			30	
Link Distance (ft)		532			295			834			1814	
Travel Time (s)		14.5			8.0			19.0			41.2	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	0%	2%	0%	0%	0%	2%	2%	0%	0%	2%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	152	106	0	57	60	0	101	304	81	85	331	85
Turn Type	pm+pt	NA		pm+pt	NA		pm+pt	NA	pm+ov	pm+pt	NA	pm+ov
Protected Phases	7	4		3	8		5	2	3	1	6	7
Permitted Phases	4			8			2		2	6		6
Detector Phase	7	4		3	8		5	2	3	1	6	7
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		22.5	24.0		9.5	24.0	22.5	9.5	24.0	9.5
Total Split (s)	16.0	24.0		24.0	32.0		13.0	30.0	24.0	12.0	29.0	16.0
Total Split (%)	17.8%	26.7%		26.7%	35.6%		14.4%	33.3%	26.7%	13.3%	32.2%	17.8%
Yellow Time (s)	3.5	4.0		3.5	4.0		3.5	4.0	3.5	3.5	4.0	3.5
All-Red Time (s)	0.0	2.0		0.0	2.0		0.0	2.0	0.0	0.0	2.0	0.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	3.5	6.0		3.5	6.0		3.5	6.0	3.5	3.5	6.0	3.5
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag	Lead	Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None		None	C-Max	None	None	C-Max	None
Act Effect Green (s)	21.0	9.3		15.2	6.5		59.2	50.5	64.2	58.6	50.1	67.9
Actuated g/C Ratio	0.23	0.10		0.17	0.07		0.66	0.56	0.71	0.65	0.56	0.75
v/c Ratio	0.49	0.41		0.21	0.35		0.14	0.15	0.07	0.11	0.17	0.07
Control Delay	32.4	12.9		26.3	17.1		9.7	18.7	1.7	6.6	12.3	0.6
Queue Delay	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	32.4	12.9		26.3	17.1		9.7	18.7	1.7	6.6	12.3	0.6
LOS	C	B		C	B		A	B	A	A	B	A
Approach Delay		24.4			21.6			14.0			9.3	
Approach LOS		C			C			B			A	
Queue Length 50th (ft)	70	1		25	1		22	59	0	15	50	0
Queue Length 95th (ft)	115	46		51	37		m59	111	m13	36	86	8

Lanes, Volumes, Timings

9: Eola Road & Commercial Full Access/Lincoln Crossing Full Access

03/18/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)		452			215			754			1734	
Turn Bay Length (ft)							185		115	115		185
Base Capacity (vph)	333	401		462	509		767	1984	1362	781	1971	1232
Starvation Cap Reductn	0	0		0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0		0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0		0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.46	0.26		0.12	0.12		0.13	0.15	0.06	0.11	0.17	0.07

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.49

Intersection Signal Delay: 14.9

Intersection LOS: B

Intersection Capacity Utilization 42.0%

ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 9: Eola Road & Commercial Full Access/Lincoln Crossing Full Access

 Ø1	 Ø2 (R)	 Ø3	 Ø4
12 s	30 s	24 s	24 s
 Ø5	 Ø6 (R)	 Ø7	 Ø8
13 s	29 s	16 s	32 s

HCM 6th TWSC
5: Wolf's Crossing Road & Hoffman Blvd.

03/18/2021

Intersection

Int Delay, s/veh 1

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	32	392	698	45	20	19
Future Vol, veh/h	32	392	698	45	20	19
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	100	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	8	5	0	0	0
Mvmt Flow	34	413	735	47	21	20

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	782	0	0 1240 759
Stage 1	-	-	- - 759 -
Stage 2	-	-	- - 481 -
Critical Hdwy	4.1	-	- - 6.4 6.2
Critical Hdwy Stg 1	-	-	- - 5.4 -
Critical Hdwy Stg 2	-	-	- - 5.4 -
Follow-up Hdwy	2.2	-	- - 3.5 3.3
Pot Cap-1 Maneuver	845	-	- - 195 410
Stage 1	-	-	- - 466 -
Stage 2	-	-	- - 626 -
Platoon blocked, %		-	- -
Mov Cap-1 Maneuver	845	-	- - 187 410
Mov Cap-2 Maneuver	-	-	- - 187 -
Stage 1	-	-	- - 447 -
Stage 2	-	-	- - 626 -

Approach	EB	WB	SB
HCM Control Delay, s	0.7	0	21.9
HCM LOS			C

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	845	-	-	-	254
HCM Lane V/C Ratio	0.04	-	-	-	0.162
HCM Control Delay (s)	9.4	-	-	-	21.9
HCM Lane LOS	A	-	-	-	C
HCM 95th %tile Q(veh)	0.1	-	-	-	0.6

Intersection						
Int Delay, s/veh	0.7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations						
Traffic Vol, veh/h	415	33	47	670	10	9
Future Vol, veh/h	415	33	47	670	10	9
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	100	-	0	-
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	437	35	49	705	11	9
Major/Minor	Major1	Major2		Minor1		
Conflicting Flow All	0	0	472	0	1258	455
Stage 1	-	-	-	-	455	-
Stage 2	-	-	-	-	803	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1090	-	189	605
Stage 1	-	-	-	-	639	-
Stage 2	-	-	-	-	441	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	-	-	1090	-	180	605
Mov Cap-2 Maneuver	-	-	-	-	180	-
Stage 1	-	-	-	-	639	-
Stage 2	-	-	-	-	421	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.6		19.4	
HCM LOS					C	
Minor Lane/Major Mvmt	NBLn1	EBT	EBR	WBL	WBT	
Capacity (veh/h)	270	-	-	1090	-	
HCM Lane V/C Ratio	0.074	-	-	0.045	-	
HCM Control Delay (s)	19.4	-	-	8.5	-	
HCM Lane LOS	C	-	-	A	-	
HCM 95th %tile Q(veh)	0.2	-	-	0.1	-	

HCM 6th TWSC
7: US 30 & Commercial RI/RO

03/18/2021

Intersection

Int Delay, s/veh 1.1

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	104	1027	144	0	1218
Future Vol, veh/h	0	104	1027	144	0	1218
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	-	0	-	240	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	2	0	0	2
Mvmt Flow	0	109	1081	152	0	1282

Major/Minor	Minor1	Major1	Major2
Conflicting Flow All	-	1081	0
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	-	6.2	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	-	3.3	-
Pot Cap-1 Maneuver	0	267	-
Stage 1	0	-	-
Stage 2	0	-	-
Platoon blocked, %		-	-
Mov Cap-1 Maneuver	-	267	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-

Approach	WB	NB	SB
HCM Control Delay, s	27.5	0	0
HCM LOS	D		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBT
Capacity (veh/h)	-	-	267
HCM Lane V/C Ratio	-	-	0.41
HCM Control Delay (s)	-	-	27.5
HCM Lane LOS	-	-	D
HCM 95th %tile Q(veh)	-	-	1.9

HCM 6th TWSC
8: US 30 & Lincoln Prairie Full Access

03/18/2021

Intersection						
Int Delay, s/veh	2.9					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	38	51	1426	65	128	1402
Future Vol, veh/h	38	51	1426	65	128	1402
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	240	240	-
Veh in Median Storage, #	1	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	40	54	1501	68	135	1476
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	3247	1501	0	0	1569	0
Stage 1	1501	-	-	-	-	-
Stage 2	1746	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	~ 10	150	-	-	420	-
Stage 1	204	-	-	-	-	-
Stage 2	154	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	~ 7	150	-	-	420	-
Mov Cap-2 Maneuver	66	-	-	-	-	-
Stage 1	204	-	-	-	-	-
Stage 2	105	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	75.9	0		1.5		
HCM LOS	F					
Minor Lane/Major Mvmt	NBT	NBR	WBLn1	WBLn2	SBL	SBT
Capacity (veh/h)	-	-	66	150	420	-
HCM Lane V/C Ratio	-	-	0.606	0.358	0.321	-
HCM Control Delay (s)	-	-	121.7	41.8	17.6	-
HCM Lane LOS	-	-	F	E	C	-
HCM 95th %tile Q(veh)	-	-	2.6	1.5	1.4	-
Notes						
~: Volume exceeds capacity		\$: Delay exceeds 300s		+: Computation Not Defined		*: All major volume in platoon

HCM 6th TWSC
11: 111th Street & Full Access Drive

03/18/2021

Intersection

Int Delay, s/veh 6.7

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	116	308	650	76	91	158
Future Vol, veh/h	116	308	650	76	91	158
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	185	-	-	185	0	-
Veh in Median Storage, #	-	0	0	-	1	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	2	2	0	0	0
Mvmt Flow	122	324	684	80	96	166

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	764	0	0 1252 684
Stage 1	-	-	- 684 -
Stage 2	-	-	- 568 -
Critical Hdwy	4.1	-	- 6.4 6.2
Critical Hdwy Stg 1	-	-	- 5.4 -
Critical Hdwy Stg 2	-	-	- 5.4 -
Follow-up Hdwy	2.2	-	- 3.5 3.3
Pot Cap-1 Maneuver	858	-	- 192 452
Stage 1	-	-	- 505 -
Stage 2	-	-	- 571 -
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	858	-	- 165 452
Mov Cap-2 Maneuver	-	-	- 298 -
Stage 1	-	-	- 433 -
Stage 2	-	-	- 571 -

Approach	EB	WB	SB
HCM Control Delay, s	2.7	0	33.1
HCM LOS			D

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	858	-	-	-	380
HCM Lane V/C Ratio	0.142	-	-	-	0.69
HCM Control Delay (s)	9.9	-	-	-	33.1
HCM Lane LOS	A	-	-	-	D
HCM 95th %tile Q(veh)	0.5	-	-	-	5

Lanes, Volumes, Timings

1: US 30 & Wolf's Crossing Road

03/19/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	17	254	115	93	395	231	179	938	14	160	1010	20
Future Volume (vph)	17	254	115	93	395	231	179	938	14	160	1010	20
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	100		215	280		280	130		0	282		0
Storage Lanes	0		1	1		1	1		0	1		0
Taper Length (ft)	150			255			112			180		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt			0.850			0.850		0.998			0.997	
Flt Protected		0.997		0.950			0.950			0.950		
Satd. Flow (prot)	0	1894	1615	1805	1980	1553	1805	1790	0	1752	1806	0
Flt Permitted		0.754		0.219			0.062			0.062		
Satd. Flow (perm)	0	1433	1615	416	1980	1553	118	1790	0	114	1806	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			100			93		1			1	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		1099			2118			1789			736	
Travel Time (s)		25.0			48.1			40.7			16.7	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	0%	0%	0%	1%	4%	0%	6%	0%	3%	5%	0%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	285	121	98	416	243	188	1002	0	168	1084	0
Turn Type	Perm	NA	pm+ov	pm+pt	NA	pm+ov	pm+pt	NA		pm+pt	NA	
Protected Phases		4	5	3	8	1	5	2		1	6	
Permitted Phases	4		4	8		8	2			6		
Detector Phase	4	4	5	3	8	1	5	2		1	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0		5.0	5.0	
Minimum Split (s)	24.0	24.0	9.5	9.5	24.0	9.5	9.5	24.0		9.5	24.0	
Total Split (s)	28.0	28.0	12.0	10.0	38.0	12.0	12.0	70.0		12.0	70.0	
Total Split (%)	23.3%	23.3%	10.0%	8.3%	31.7%	10.0%	10.0%	58.3%		10.0%	58.3%	
Yellow Time (s)	4.0	4.0	3.5	3.5	4.0	3.5	3.5	4.0		3.5	4.0	
All-Red Time (s)	2.0	2.0	0.0	0.0	2.0	0.0	0.0	2.0		0.0	2.0	
Lost Time Adjust (s)		0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Lost Time (s)		6.0	3.5	3.5	6.0	3.5	3.5	6.0		3.5	6.0	
Lead/Lag	Lag	Lag	Lead	Lead		Lead	Lead	Lag		Lead	Lag	
Lead-Lag Optimize?	Yes	Yes	Yes	Yes		Yes	Yes	Yes		Yes	Yes	
Recall Mode	None	None	None	None	None	None	None	Max		None	Max	
Act Effect Green (s)		22.0	36.5	34.5	32.0	46.5	75.0	64.0		75.0	64.0	
Actuated g/C Ratio		0.18	0.30	0.29	0.27	0.39	0.62	0.53		0.62	0.53	
v/c Ratio		1.09	0.22	0.51	0.79	0.37	0.97	1.05		0.90	1.13	
Control Delay		127.2	9.6	42.2	52.9	17.6	87.8	71.3		71.4	98.2	
Queue Delay		0.0	0.0	0.0	0.0	0.0	0.0	0.0		0.0	0.0	
Total Delay		127.2	9.6	42.2	52.9	17.6	87.8	71.3		71.4	98.2	
LOS		F	A	D	D	B	F	E		E	F	
Approach Delay		92.1			40.2			73.9			94.6	
Approach LOS		F			D			E			F	
Queue Length 50th (ft)		~248	12	57	300	79	93	~846		78	~973	
Queue Length 95th (ft)		#422	56	102	#445	147	#245	#1102		#213	#1234	

Lanes, Volumes, Timings

1: US 30 & Wolf's Crossing Road

03/19/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	1019			2038			1709			656		
Turn Bay Length (ft)	215			280			130			282		
Base Capacity (vph)	262			560			193			187		
Starvation Cap Reductn	0			0			0			0		
Spillback Cap Reductn	0			0			0			0		
Storage Cap Reductn	0			0			0			0		
Reduced v/c Ratio	1.09			0.22			0.51			0.79		

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Natural Cycle: 140

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.13

Intersection Signal Delay: 76.1

Intersection LOS: E

Intersection Capacity Utilization 116.7%

ICU Level of Service H

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: US 30 & Wolf's Crossing Road

			
Ø1	Ø2	Ø3	Ø4
12 s	70 s	10 s	28 s
			
Ø5	Ø6	Ø8	
12 s	70 s	38 s	

Lanes, Volumes, Timings

3: US 30 & Eola Road

03/19/2021

						
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Volume (vph)	445	23	1148	329	133	1085
Future Volume (vph)	445	23	1148	329	133	1085
Ideal Flow (vphpl)	1900	1900	2000	1900	1900	2000
Storage Length (ft)	310	0		240	240	
Storage Lanes	1	1		1	1	
Taper Length (ft)	200				240	
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.850		0.850		
Flt Protected	0.950				0.950	
Satd. Flow (prot)	1787	1615	1905	1599	1805	1905
Flt Permitted	0.950				0.053	
Satd. Flow (perm)	1787	1615	1905	1599	101	1905
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)		22		228		
Link Speed (mph)	30		30			30
Link Distance (ft)	660		2004			828
Travel Time (s)	15.0		45.5			18.8
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	1%	0%	5%	1%	0%	5%
Shared Lane Traffic (%)						
Lane Group Flow (vph)	468	24	1208	346	140	1142
Turn Type	Prot	Perm	NA	Perm	pm+pt	NA
Protected Phases	8		2		1	6
Permitted Phases		8		2	6	
Detector Phase	8	8	2	2	1	6
Switch Phase						
Minimum Initial (s)	5.0	5.0	5.0	5.0	5.0	5.0
Minimum Split (s)	24.0	24.0	24.0	24.0	9.5	24.0
Total Split (s)	32.0	32.0	78.0	78.0	10.0	88.0
Total Split (%)	26.7%	26.7%	65.0%	65.0%	8.3%	73.3%
Yellow Time (s)	4.0	4.0	4.0	4.0	3.5	4.0
All-Red Time (s)	2.0	2.0	2.0	2.0	0.0	2.0
Lost Time Adjust (s)	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	6.0	6.0	6.0	6.0	3.5	6.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Recall Mode	None	None	C-Max	C-Max	None	C-Max
Act Effect Green (s)	26.0	26.0	72.0	72.0	84.5	82.0
Actuated g/C Ratio	0.22	0.22	0.60	0.60	0.70	0.68
v/c Ratio	1.21	0.07	1.06	0.33	0.86	0.88
Control Delay	156.8	15.9	67.9	4.7	64.4	24.7
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	156.8	15.9	67.9	4.7	64.4	24.7
LOS	F	B	E	A	E	C
Approach Delay	149.9		53.8			29.1
Approach LOS	F		D			C
Queue Length 50th (ft)	~442	1	~1026	37	54	635
Queue Length 95th (ft)	#650	24	#1285	83	#173	913

Lanes, Volumes, Timings

3: US 30 & Eola Road

03/19/2021



Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Internal Link Dist (ft)	580		1924			748
Turn Bay Length (ft)	310			240	240	
Base Capacity (vph)	387	367	1143	1050	163	1301
Starvation Cap Reductn	0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0	0
Reduced v/c Ratio	1.21	0.07	1.06	0.33	0.86	0.88

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 120

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 140

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.21

Intersection Signal Delay: 58.5

Intersection LOS: E

Intersection Capacity Utilization 102.8%

ICU Level of Service G

Analysis Period (min) 15

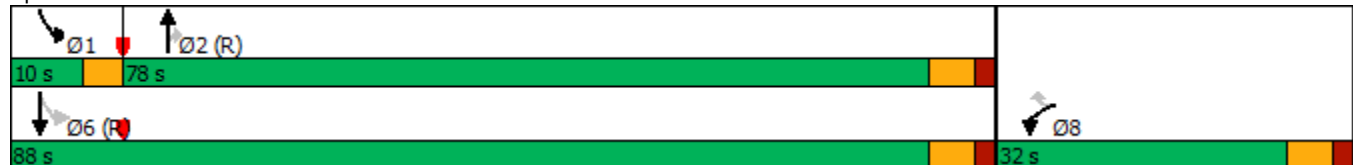
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 3: US 30 & Eola Road



Lanes, Volumes, Timings
4: US 30 & 111th Street

03/19/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	112	88	7	79	230	499	20	880	78	258	1026	156
Future Volume (vph)	112	88	7	79	230	499	20	880	78	258	1026	156
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	1900	1900	1900	2000	1900
Storage Length (ft)	240		0	240		240	125		0	190		265
Storage Lanes	1		0	1		1	1		0	1		1
Taper Length (ft)	240			240			150			160		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Frt		0.989				0.850		0.988				0.850
Flt Protected	0.950			0.950			0.950			0.950		
Satd. Flow (prot)	1805	1862	0	1128	2000	1509	1805	1664	0	1736	1770	1583
Flt Permitted	0.243			0.692			0.062			0.059		
Satd. Flow (perm)	462	1862	0	822	2000	1509	118	1664	0	108	1770	1583
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		3				113		5				164
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		2372			910			1113			1927	
Travel Time (s)		53.9			20.7			25.3			43.8	
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	0%	1%	0%	60%	0%	7%	0%	11%	33%	4%	13%	2%
Shared Lane Traffic (%)												
Lane Group Flow (vph)	118	100	0	83	242	525	21	1008	0	272	1080	164
Turn Type	pm+pt	NA		pm+pt	NA	pm+ov	pm+pt	NA		pm+pt	NA	pm+ov
Protected Phases	7	4		3	8	1	5	2		1	6	7
Permitted Phases	4			8		8	2			6		6
Detector Phase	7	4		3	8	1	5	2		1	6	7
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0	5.0	5.0	5.0		5.0	5.0	5.0
Minimum Split (s)	9.5	24.0		9.5	24.0	9.5	9.5	24.0		9.5	24.0	9.5
Total Split (s)	10.0	24.0		10.0	24.0	18.0	10.0	68.0		18.0	76.0	10.0
Total Split (%)	8.3%	20.0%		8.3%	20.0%	15.0%	8.3%	56.7%		15.0%	63.3%	8.3%
Yellow Time (s)	3.5	4.0		3.5	4.0	3.5	3.5	4.0		3.5	4.0	3.5
All-Red Time (s)	0.0	2.0		0.0	2.0	0.0	0.0	2.0		0.0	2.0	0.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Lost Time (s)	3.5	6.0		3.5	6.0	3.5	3.5	6.0		3.5	6.0	3.5
Lead/Lag	Lead	Lag		Lead	Lag	Lead	Lead	Lag		Lead	Lag	Lead
Lead-Lag Optimize?	Yes	Yes		Yes	Yes	Yes	Yes	Yes		Yes	Yes	Yes
Recall Mode	None	None		None	None	None	None	Max		None	Max	None
Act Effect Green (s)	26.8	19.1		26.1	17.1	37.6	70.5	62.0		82.5	76.1	88.6
Actuated g/C Ratio	0.23	0.16		0.22	0.14	0.32	0.59	0.52		0.69	0.64	0.74
v/c Ratio	0.67	0.33		0.42	0.85	0.95	0.14	1.16		1.00	0.95	0.13
Control Delay	56.5	47.6		44.7	75.4	59.5	8.7	113.2		89.4	39.9	1.2
Queue Delay	0.0	0.0		0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0
Total Delay	56.5	47.6		44.7	75.4	59.5	8.7	113.2		89.4	39.9	1.2
LOS	E	D		D	E	E	A	F		F	D	A
Approach Delay		52.4			62.6			111.1			44.6	
Approach LOS		D			E			F			D	
Queue Length 50th (ft)	73	68		52	184	324	5	~932		~164	655	0
Queue Length 95th (ft)	#141	123		100	#313	#547	13	#1189		#345	#1163	20

Lanes, Volumes, Timings

4: US 30 & 111th Street

03/19/2021

												
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Internal Link Dist (ft)	2292			830			1033			1847		
Turn Bay Length (ft)	240			240		240	125			190		265
Base Capacity (vph)	177	302		196	302	553	162	869		272	1131	1219
Starvation Cap Reductn	0	0		0	0	0	0	0		0	0	0
Spillback Cap Reductn	0	0		0	0	0	0	0		0	0	0
Storage Cap Reductn	0	0		0	0	0	0	0		0	0	0
Reduced v/c Ratio	0.67	0.33		0.42	0.80	0.95	0.13	1.16		1.00	0.95	0.13

Intersection Summary

Area Type: Other

Cycle Length: 120

Actuated Cycle Length: 119.1

Natural Cycle: 150

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 1.16

Intersection Signal Delay: 68.2

Intersection LOS: E

Intersection Capacity Utilization 99.8%

ICU Level of Service F

Analysis Period (min) 15

~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.

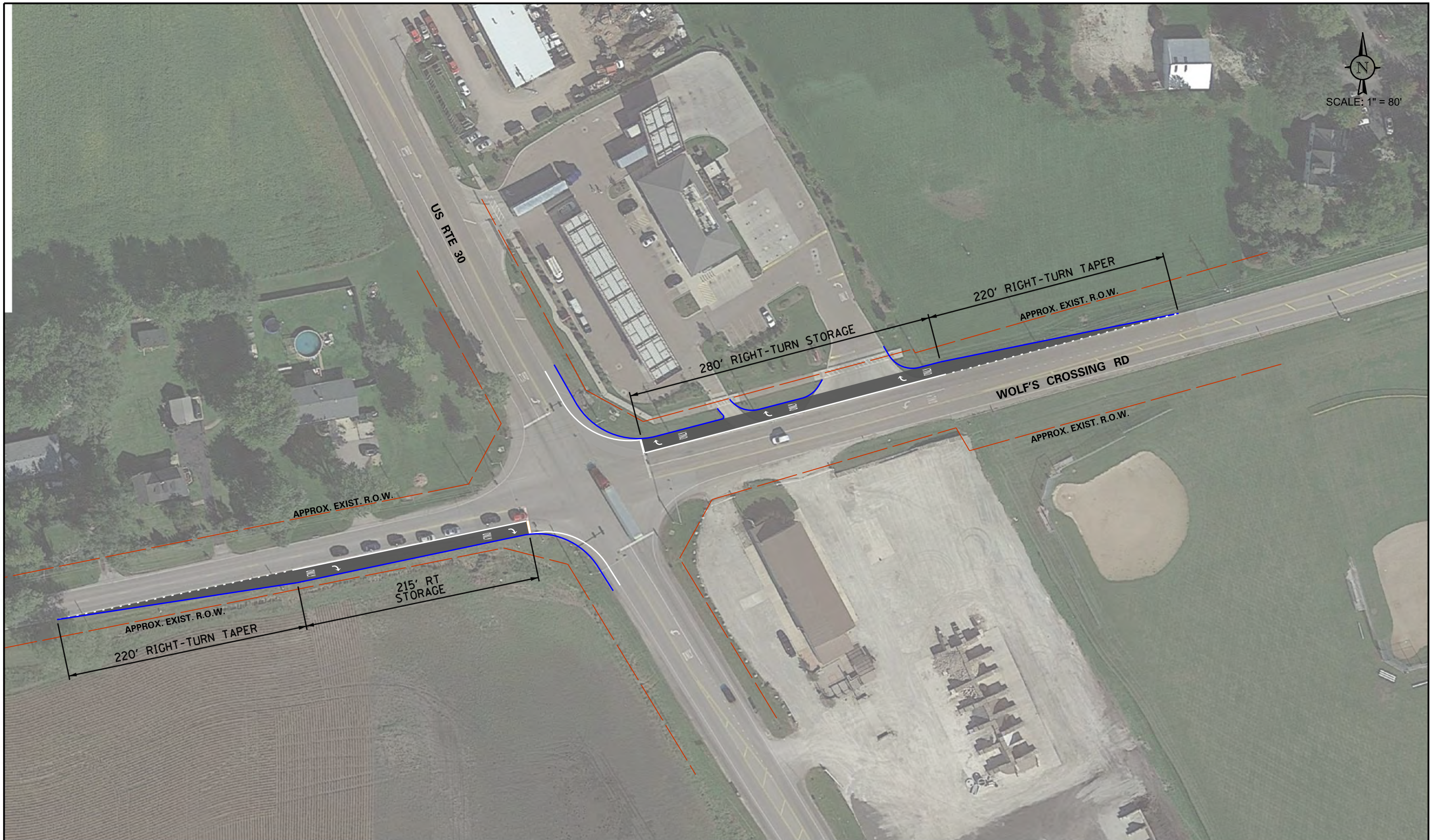
95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 4: US 30 & 111th Street

 Ø1	 Ø2	 Ø3	 Ø4
18 s	58 s	10 s	24 s
 Ø5	 Ø6	 Ø7	 Ø8
10 s	76 s	10 s	24 s

Preliminary Regional Geometric Improvements



LINCOLN PRAIRIE
AURORA, ILLINOIS

**PROPOSED PRELIMINARY IMPROVEMENTS
(ADDITION OF RIGHT-TURN LANES ON WOLF'S CROSSING RD)**

DRAWN: MD
DATE: 02-09-21
PROJECT # 21-001
EXHIBIT: A

CHECKED: JM
REV:



